Precision Structural Engineering, Inc.

STRUCTURAL ENGINEERING CALCULATIONS

PROJECT: Stainless Cable & Railing – Fascia Mount

PROJECT LOCATION: Tennessee

PSE PROJECT NUMBER: Stainless Cable & Railing
216-1 Railing

DATE: July 23, 2018

7-26-18

BY: Nabil Taha, Ph.D., P.E.

NABIL TAHA
REGISTERED ENGINEER
AGRICULTURE
COMMERCE
No. 116974
STATE OF TENNESSEE

250-A Main Street
Klamath Falls, OR. 97601

E-Mail: info@structure1.com
Web: www.structure1.com

ph. (541) 850-6300
fax (541) 850-6233

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250-A Main Street
Klamath Falls, OR. 97601
E-Mail: info@structure1.com
Web: www.structure1.com
ph. (541) 850-6300
fax (541) 850-6233

Precision Structural Engineering, Inc.

References:

1- Literature:

a. 2015 International Building Code (IBC)

b. Aluminum Design Manual ADMI-15, The Aluminum Association.

c. National Design Specifications for Wood Construction, 2015 Edition

2- Software:

a. RISA 3D Version 14.0.1
RISA Technologies,
26212 Dimension Dr. Suite 200

250-A Main Street
Klamath Falls, OR. 97601
E-Mail: info@structure1.com
Web: www.structure1.com
ph. (541) 850-6300
fax (541) 850-6233

Design Criteria:

1- Location: Washington & California

2- Live Load on Handrail & guards:

a. Uniform Distributed load 50 p/f
b. Single Concentrated load 200 lbs

**Other criteria assumed as stated in design calculations.

250-A Main Street
Klamath Falls, OR. 97601
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fax (541) 850-6233

Precision Structural Engineering, Inc.
250 Main Street, Suite A • Klamath Falls, OR 97601
Tel (541) 850-6300 • FAX (541) 850-6233

Medford Office
836 Mason Way (off Sage Road) • Medford, OR 97501
Tel (541) 858-8500

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PROJECT NO. Stainless 216-1 SHEET ______ OF ______
PROJECT NAME ______ DESIGNED BY AF DATE 1/19/17
SUBJECT Fascia Mount CHECKED BY NT DATE 1-26-17

Conclusion

Fascia mount with bracket

* Bracket size, 316 SS, minimum size [4.01" * 6.75" * 0.39"]

* Minimum Anchor bolt or lag screw size:

  1- 4- 3/8" ϕ w/min 4" Embed, Red head ITW wedge

  2- 4- 3/8" ϕ w/min 4" Embed, Red head LDT

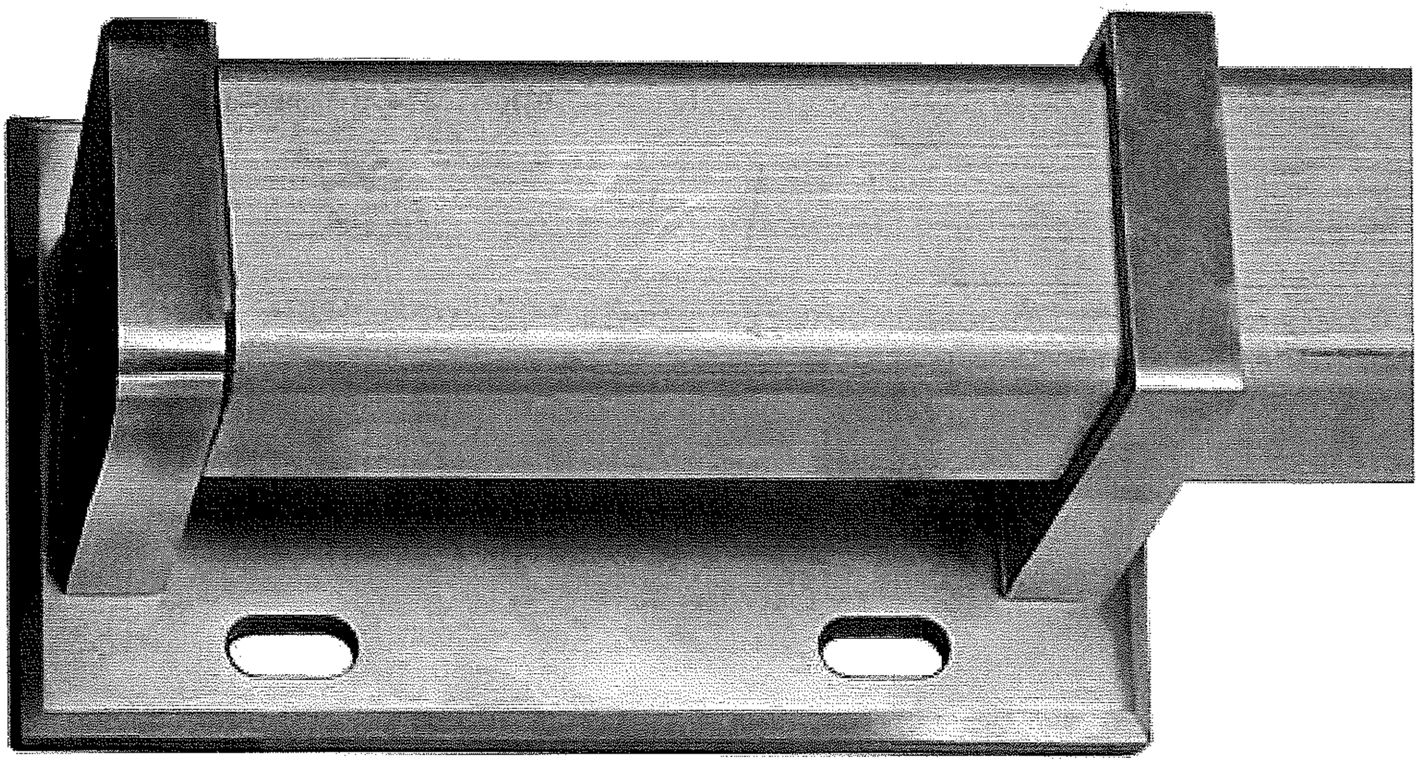
  3- 4- 3/8" ϕ lag screw w/min 5" Embed

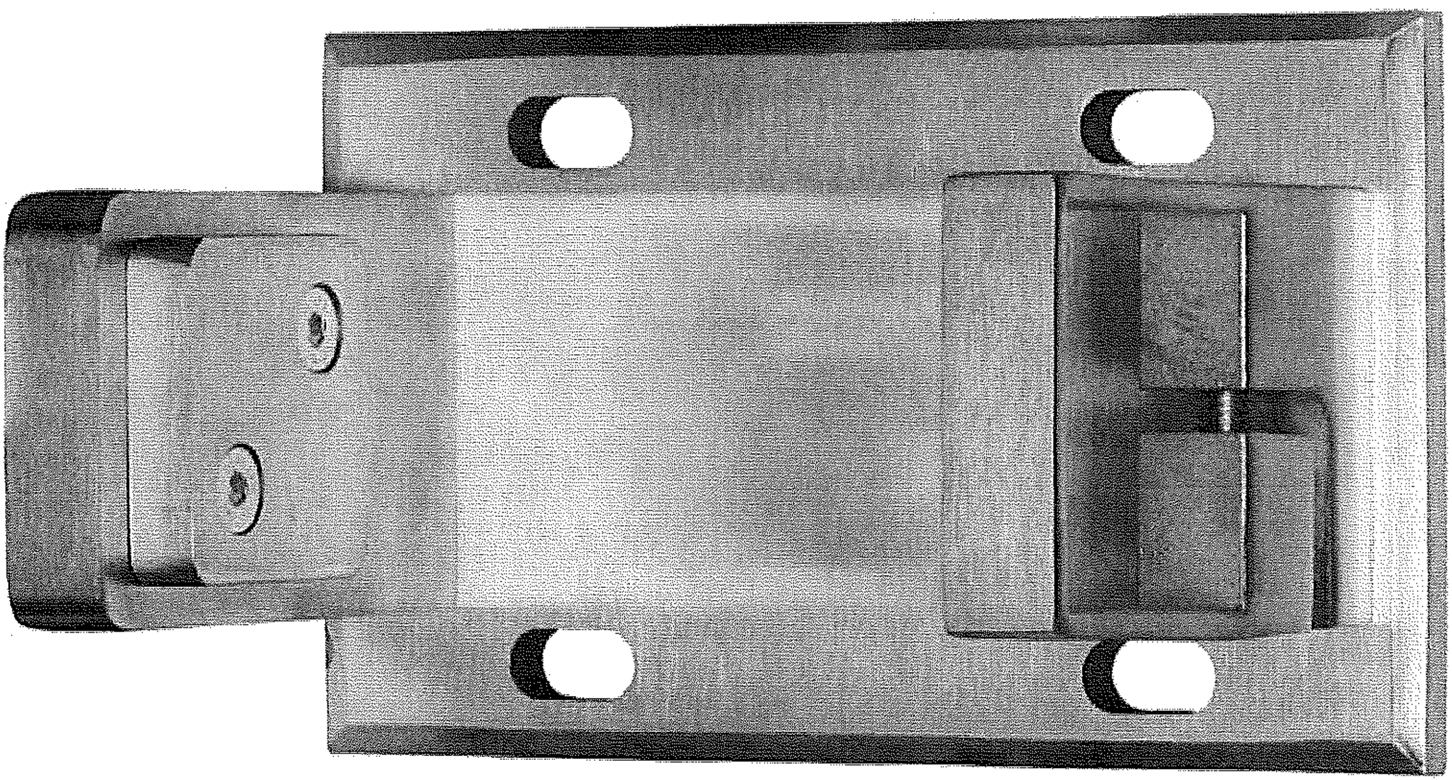
Fascia Mount with no bracket

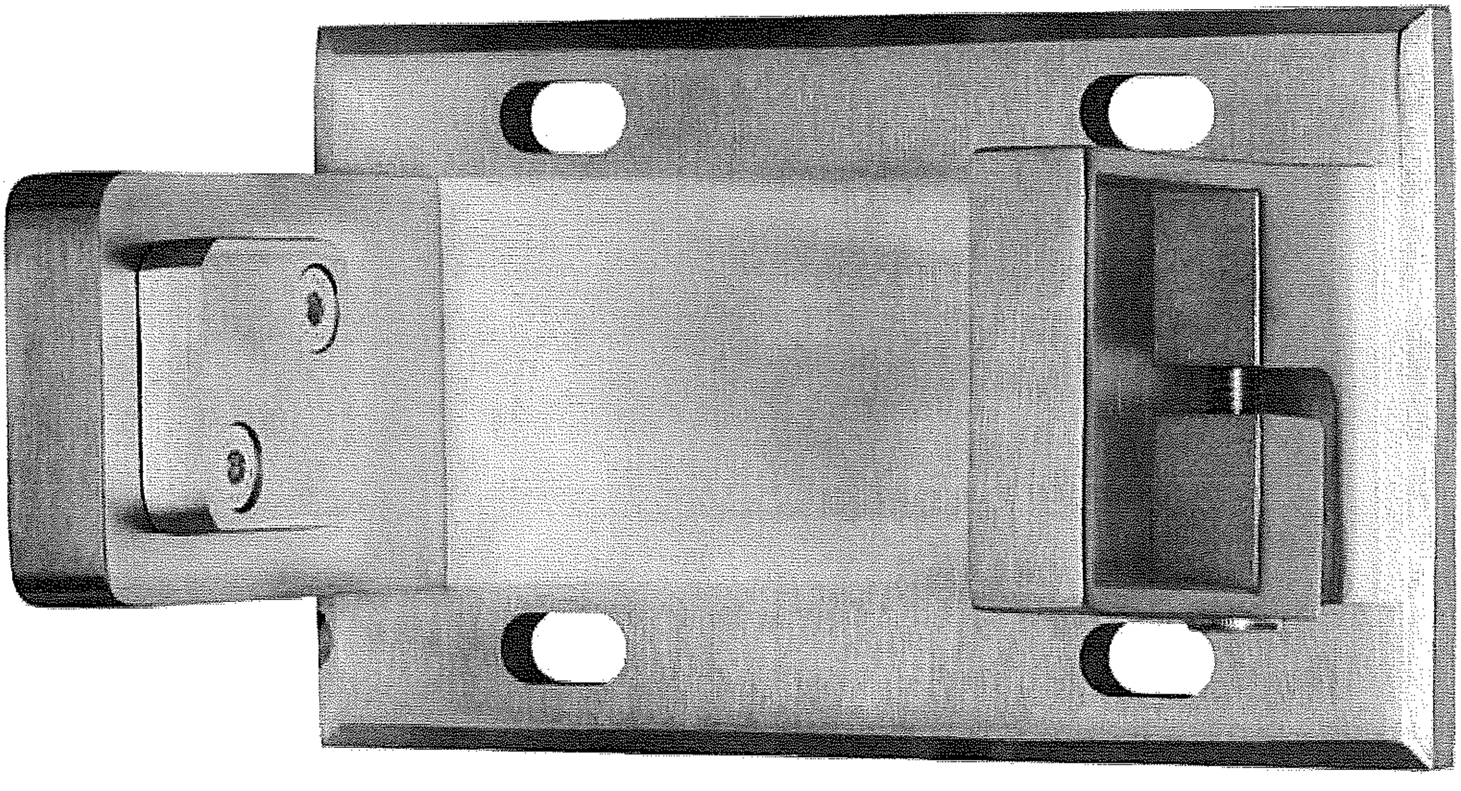
1- 2- 1/2" ϕ w/min 4" Embed, 7" apart

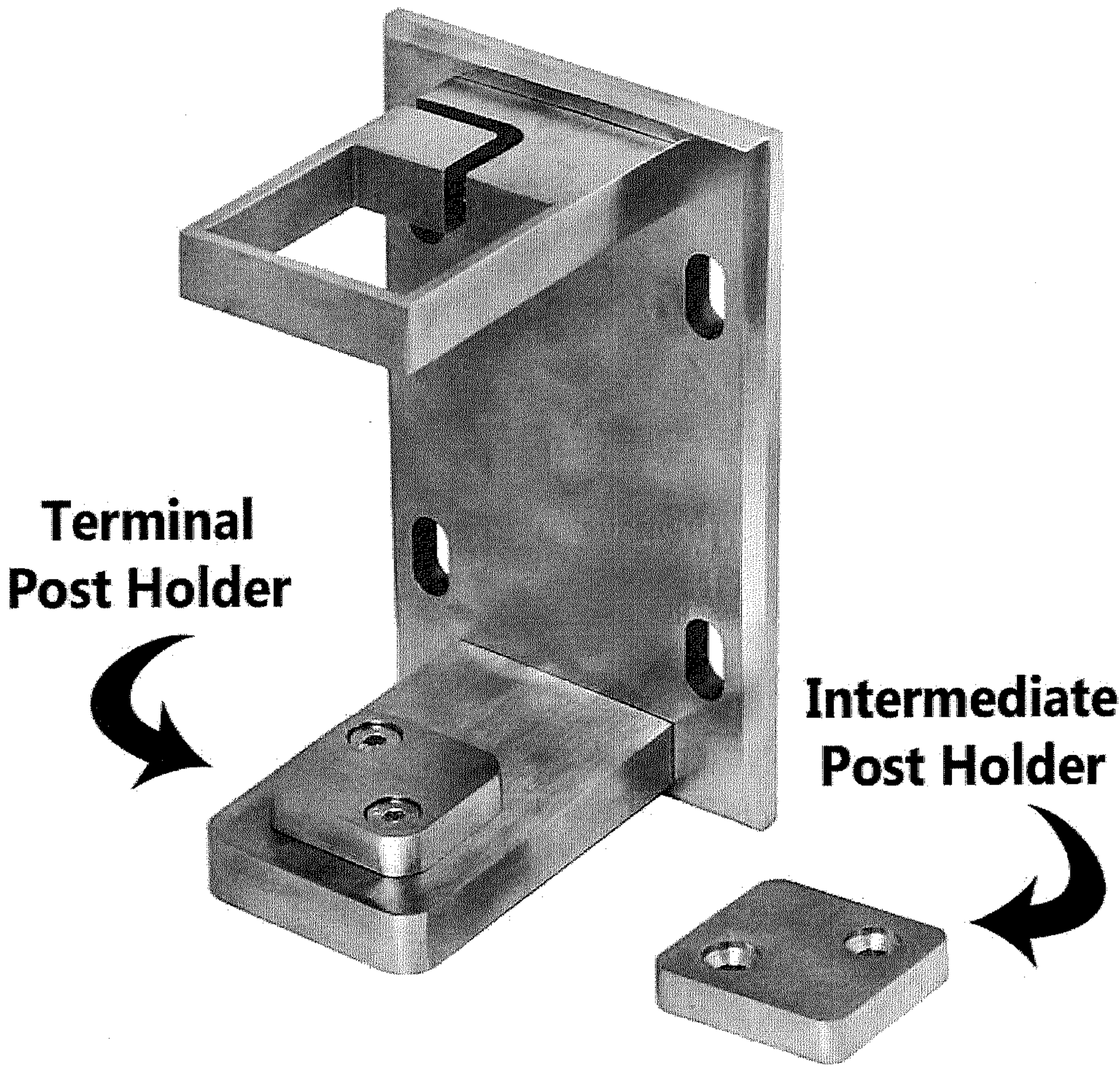
2- 2- 1/2" ϕ lag screw w/min 6" Embed, 7" apart

3- 2- 1/2" ϕ thru-bolt, Hex bolt

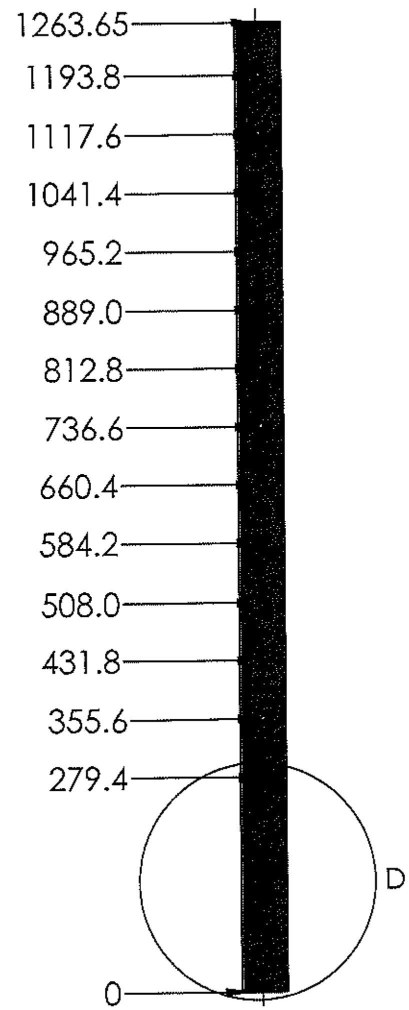


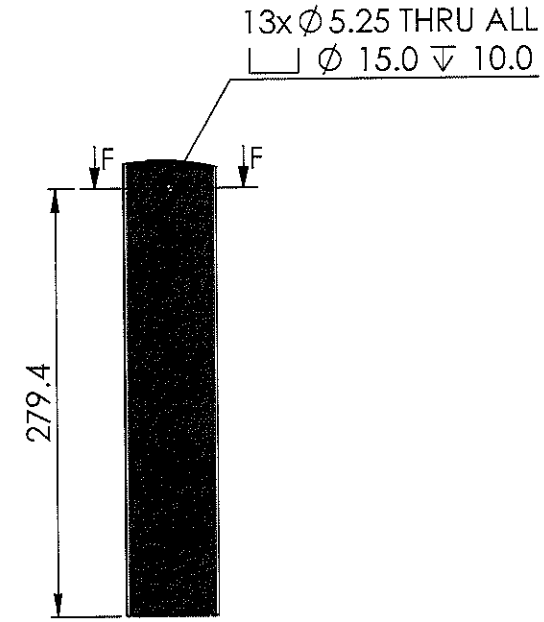




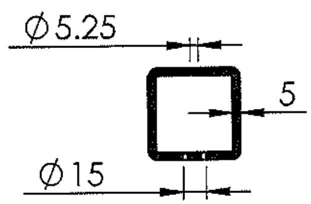


Terminal
Post Holder
Intermediate
Post Holder

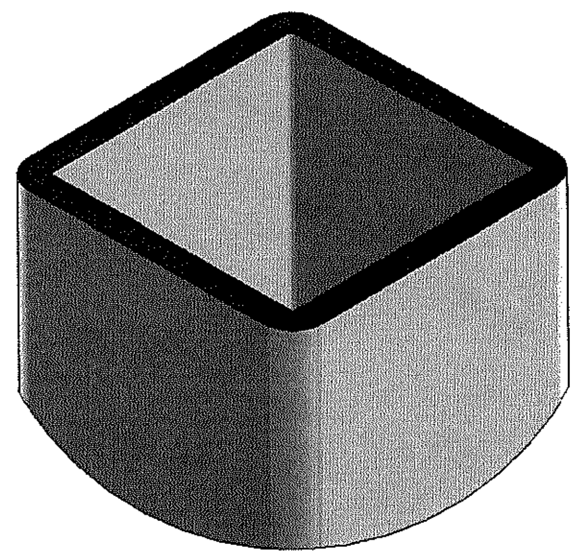




DETAIL D
SCALE 2 : 9



SECTION F-F
SCALE 2 : 9



NOTES : UNLESS OTHERWISE SPECIFIED

1. DIMENSIONS ARE IN MM.
2. MACHINED SURFACES TO BE 63uIN MAX.
3. DO NOT SCALE DRAWING.
4. PART TO BE FREE OF BURRS AND SHARP EDGES.
5. ALL MATERIAL TO BE CLEAN, DEGREASED AND FREE OF ALL IMPURITIES.
6. ANY CHANGE MUST BE AUTHORIZED BY AN SC&R ENGINEER.

REV	DATE	DESCRIPTION OF CHANGE	ECN	BY.	APPR

REVISIONS

MATERIAL TYPE : 316 SS

VENDOR ID:

SC&R Stainless Cable & Railing

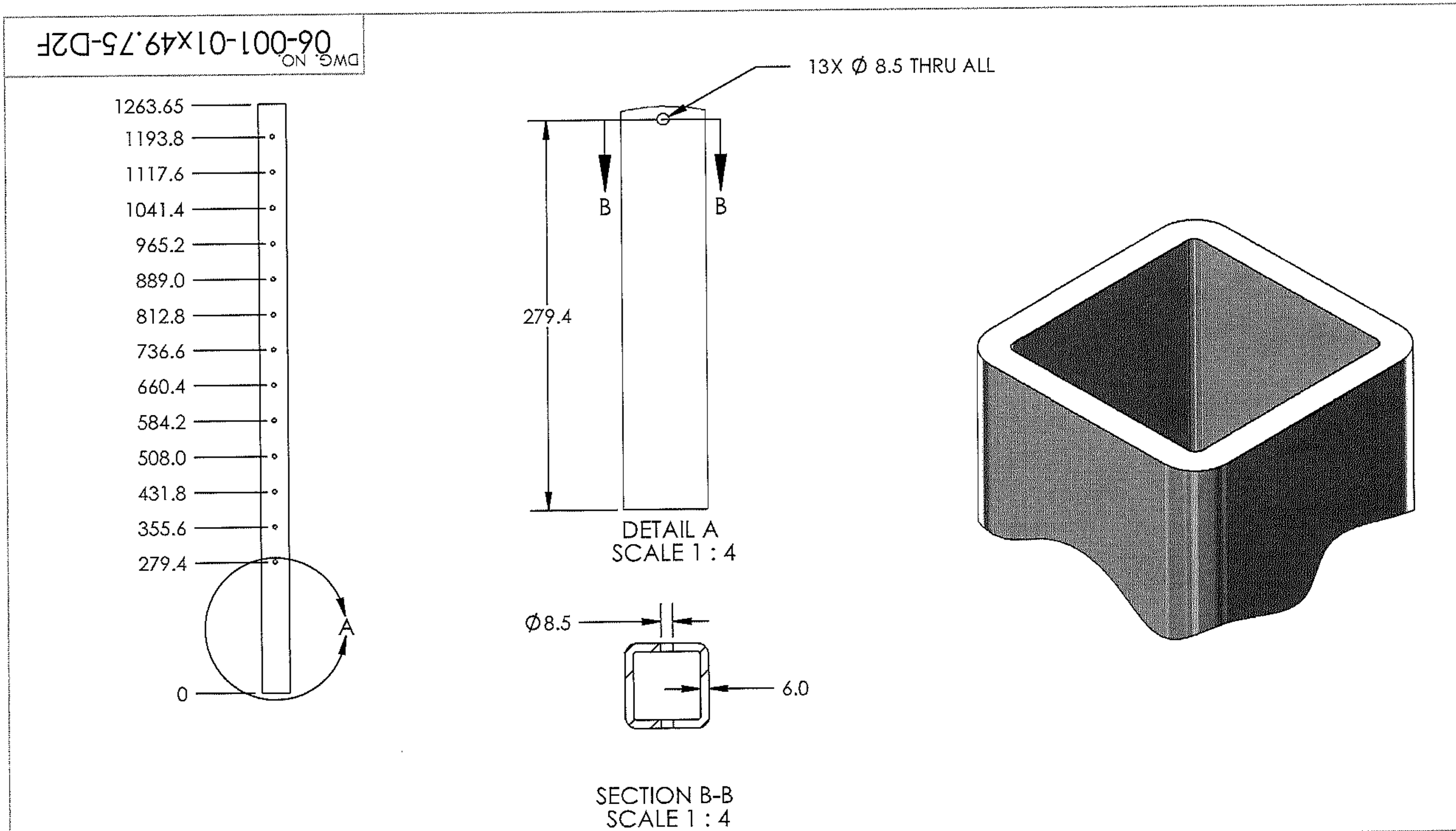
TOLERANCES
.XX ±.01 UNLESS
.XXX ±.005 OTHERWISE
ANGULAR ±0.5° SPECIFIED
FRACTIONAL ±1/32

DRAWN BY.	DATE	PROJECT
PA	12.01.13	**

TITLE : Square Tube Fascia-mount 42" Swageless Terminal Post

DRG NO. 06-001-01x49.75-D3F

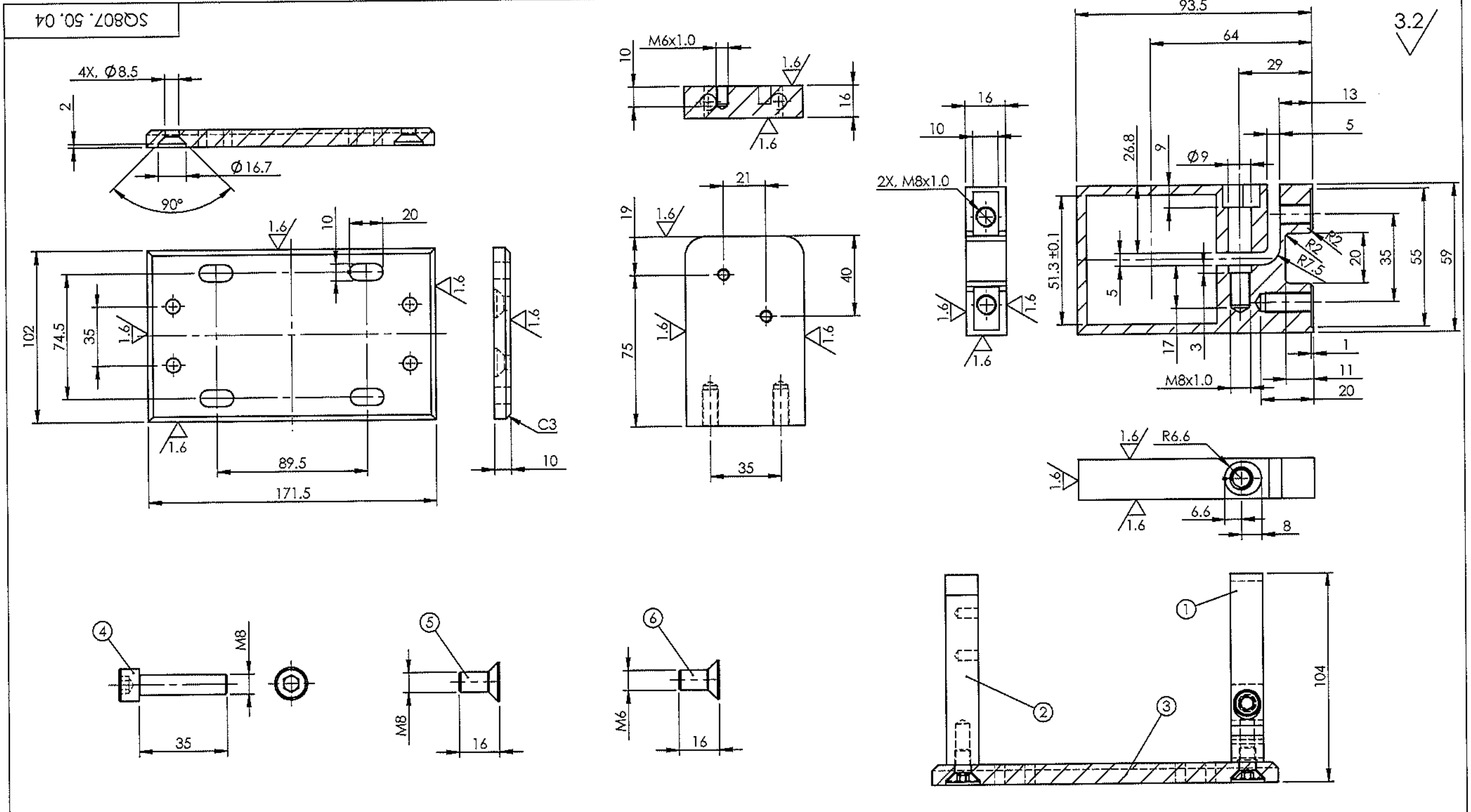
SHEET 1 OF 1



UNLESS OTHERWISE SPECIFIED:		NAME	DATE	Stainless Cable & Railing, Inc. 888-686-7245 (RAIL)
DIMENSIONS ARE IN Millimeters	DRAWN	GM		TITLE: Square Tube Fascia-Mount 42" Tensioner Terminal Post
TOLERANCES: x = 0.1, x.x = 0.1, x.xx = 0.01, x.xxx = 0.005	CHECKED			
	ENG APPR.			
	MFG APPR.			
INTERPRET GEOMETRIC TOLERANCING PER:	Q.A.			
MATERIAL 316 SS	COMMENTS:			SIZE A; DWG. NO. 06-001-01x49.75-D2F; REV A
FINISH				
DO NOT SCALE DRAWING				SCALE: 1:20 WEIGHT: SHEET 1 OF 1

NEXT ASSY	USED ON
APPLICATION	

5 4 3 2 1



S.NO.	ITEM NO.		QUANTITY
1	SQ904 50 04		1
2	SQ807 50 04-01		1
3	SQ807.50 04-2		1
4	DIN 912-M8X35		1
5	DIN 7991-M8X16		4
6	DIN 7991-M6X16		2

REV	DATE	DESCRIPTION OF CHANGE	ECN	BY.	APPR

REVISIONS

Stainless Cable & Railing

TOLERANCES
.XX ±.01 UNLESS
.XXX ±.005 OTHERWISE
ANGULAR ±0.5° SPECIFIED
FRACTIONAL ±1/32

TITLE : STAINLESS FASCIA BRACKET (SQUARE) ASSEMBLY

DRG NO. 06-003-01

SHEET 1 OF 1

DRAWN BY. | DATE | PROJECT **

FISCIA MOUNT ANALYSIS & DESIGN:

Pages 1,000 - 1,999

250-A Main Street
Klamath Falls, OR. 97601
E-Mail: info@structure1.com
Web: www.structure1.com
ph. (541) 850-6300
fax (541) 850-6233

Precision Structural Engineering, Inc.
250 Main Street, Suite A • Klamath Falls, OR 97601
Tel (541) 850-6300 • FAX (541) 850-6233

Medford Office
836 Mason Way (off Sage Road) • Medford, OR 97501
Tel (541) 858-8500

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PROJECT NO. Stainless Cable 216-1 SHEET 1000 OF ______

PROJECT NAME ______ DESIGNED BY AF DATE ______

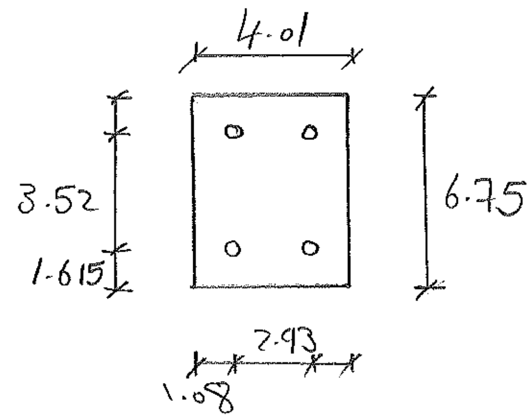
SUBJECT fascia mount CHECKED BY ______ DATE ______

fascia mount bracket design

Shear = 250 lb

M = 875 lb-ft

per page (1001)



use [4.01" * 6.75" * 0.39" SS316]

1001

STEEL COLUMN BASE PLATE ANALYSIS

Per AISC 9th Edition Manual (ASD) and "Design of Welded Structures" (O. Blodgett)

For Axial Load with or without Moment

Job Name:		Subject:			
Job Number:		Originator:		Checker:	

Input Data:

Column Size:

Select: W4x13

Column Loadings:

Axial Load, P(total) =	0.00	kips
Axial Load, P(DL) =	0.00	kips
Shear Load, V(total) =	0.25	kips
Moment @ Base, M =	0.90	ft.-kips

Design Parameters:

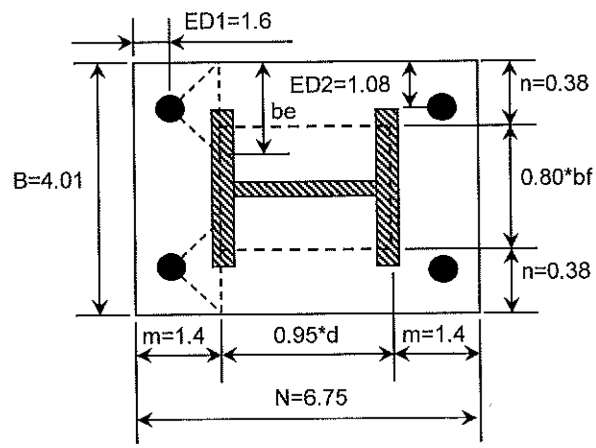
Base Plate Length, N =	6.750	in.
Base Plate Width, B =	4.010	in.
Plate Yield Stress, Fy =	30.00	ksi
Concrete Strength, f 'c =	2.500	ksi.
Bearing Area, A2 =	24.00	in.^2
Shear Coef., C =	1.85	
Coef. of Friction, μ =	0.70	

Anchor Bolt/Rod Data:

Total No. of Bolts, Nb =	4	
Bolt Diameter, db =	0.375	in.
Anchor Bolt Material =	F1554 (36)	
Bolt Edge Dist., ED1 =	1.600	in.
Bolt Edge Dist., ED2 =	1.080	in.

Column Properties:

A =	3.83	in.^2
d =	4.160	in.
tw =	0.280	in.
bf =	4.060	in.
tf =	0.345	in.



Plan

Results:

Eccentricity, Bearing Length, and Bearing Pressures:

Eccentricity, e =	Infinity	in.
Length, Xc =	1.907	in.
Fp =	0.824	ksi
fp(max) =	0.626	ksi
fp(min) =	0.000	ksi

Fp >= fp(max), O.K.

Anchor Bolt/Rod Tension and Shear:

Ft =	19.10	ksi
Ta =	2.11	k/bolt
Tb =	1.20	k/bolt

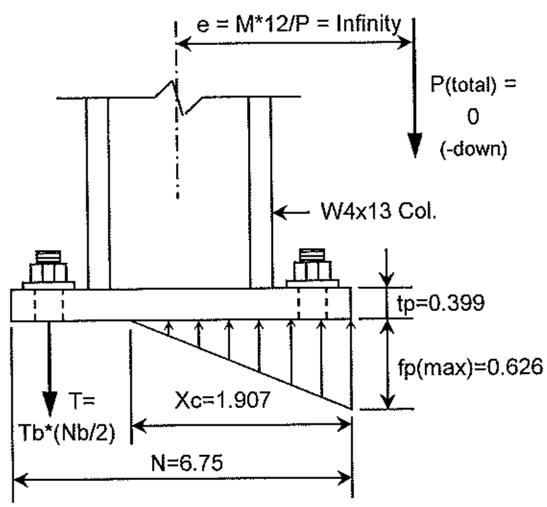
Ta >= Tb, O.K.

Fv =	9.90	ksi
Va =	1.09	k/bolt
V(bolts) =	0.25	= V(total)-1/2*μ*P(DL)
Vb =	0.06	k/bolt

Va >= Vb, O.K.

(Interaction) S.R. = 0.673 = Tb/Ta+(C*Vb)/Va

S.R. <= 1.0, O.K.



Elevation

Base Plate Thickness:

tp(req'd) = 0.399 in.

tp(min) = 0.350 in.

Suggested plate thickness for rigidity:
tp(min) >= max. of m/4 or n/4

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250 Main Street, Suite A • Klamath Falls, OR 97601
Tel (541) 850-6300 • FAX (541) 850-6233
Medford Office
836 Mason Way (off Sage Road) • Medford, OR 97501
Tel (541) 858-8500
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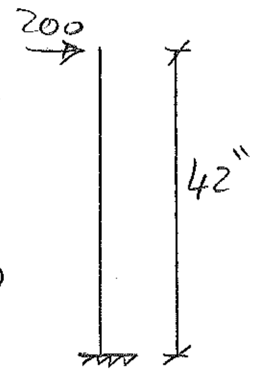
PROJECT NO. Stainless Cable 216-1 SHEET 1002 OF ______
PROJECT NAME ______ DESIGNED BY AF DATE ______
SUBJECT Fascia Mount CHECKED BY NT DATE 1-26-17

* Connection design

post height = 42"

applied load = 200 lb (Concentrated)

$M_{@base}$ = 200 (lb) * 42 (in) = 8400 lb.in ①



applied load = 50 lb/ft (Distributed)

spacing between posts (max) = 5 ft

$M_{@base}$ = 50 (lb/ft) * 5 (ft) * 42 (in) = 10,500 lb.in (2)

From ①, ②

Design moment = 10,500 lb.in
= 875 lb.ft

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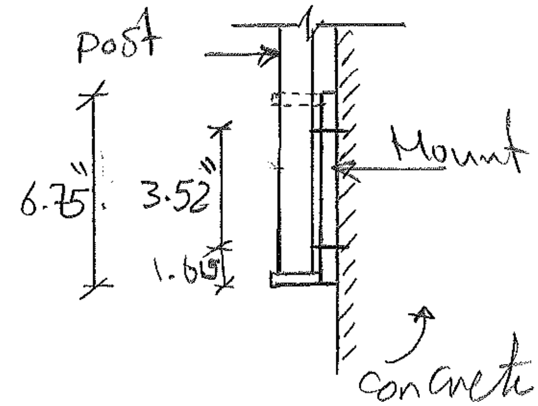
PROJECT NO. Stainless Cable 216-1 SHEET Roo3 OF ______

PROJECT NAME ______ DESIGNED BY AF DATE ______

SUBJECT Fascia Mount CHECKED BY ______ DATE ______

Fascia Mount to Concrete



Applied moment = 875 lb-ft

Bolt spacing = 3.52"

Tension induced in bolts = $\frac{M}{d}$

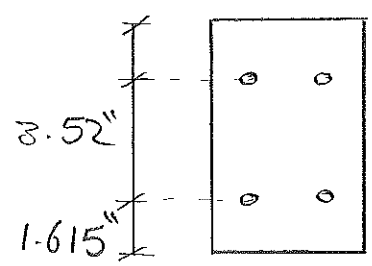
M = applied moment = 875 lb-ft

d = spacing between Tension and Compression lines

= spacing between bolts rows + edge distance

= 3.52 (in) + 1.615 (in)

= 5.135 in



∴ Tension = $\frac{875 (lb \cdot ft)}{5.135/12}$ = 2044 lb

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PROJECT NO. Stainless Cable 216-1 SHEET 1004 OF ______

PROJECT NAME ______ DESIGNED BY AF DATE ______

SUBJECT Fascia Mount CHECKED BY ______ DATE ______

Fascia mount to 2500 psi Concrete

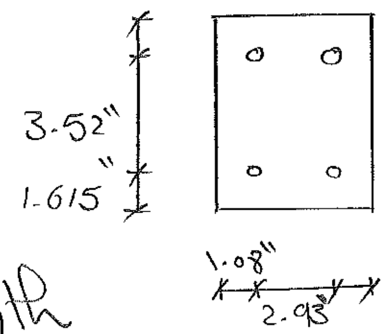
use ITW Red head Trubolt wedge

Anchor strength is based on ESR-2427
Page 1013-1029

$f'_c = 2500$ psi , min. Embed = 4"

bolt spacing = 3.52"

min. Edge distance = 2.5"



* Check Concrete break out strength

ACI 318-14

$$N_{cbg} = \frac{A_{Nc}}{A_{Nco}} \cdot \psi_{ec,N}\,\psi_{ed,N}\,\psi_{c,N}\,\psi_{cp,N}\,N_b \qquad (17.4.2.1.b)$$

$$A_{Nc} = (c_{a1} + s_1 + 1.5\,h_{ef})(2 \times 1.5\,h_{ef} + s_2)$$

$$= (2.5" + 3.52" + 1.5 \times 4)(2 \times 1.5 \times 4 + 2.93)$$

$$= 179.45\ in^2$$

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PROJECT NO. Stainless Cable 216-1 SHEET 1005 OF ______

PROJECT NAME ______ DESIGNED BY AF DATE ______

SUBJECT Fascia mount CHECKED BY ______ DATE ______

$$A_{Nco} = 9\,\overline{h_{ef}}^2 = 9 * \overline{4}^2 = 144 \text{ in}^2$$

$$\psi_{cpN} = \max\left(\frac{1.5}{4''} \text{ or } 1.5 * \frac{4''}{4''}\right) = 1.5'' > 1.0$$

use $\psi_{cp,N} = 1.0$

$$N_b = K_c \lambda_a \sqrt{f_c'}\, \overline{h_{ef}}^{1.5} \qquad (17.4.2.2a)$$

$K_c = 24$

$\lambda_a = 1.0$

$$N_b = 24\sqrt{2500} * \overline{4}^{1.5} = 9600 \text{ lb}$$

$$N_{cbg} = \frac{179.45}{144} * 1.0 * 1.0 * 1.0 * 9600 (\text{lb}) = 11963 \text{ lb}$$

∴ Concrete breakout strength of '4' bolts = 11963 lb

Bolt Tensile strength = 4200 lb

Tensile strength of 2 bolts = 2 * 4200 = 8400 lb

<

Concrete breakout strength = 11963 lb

∴ Bolts Control

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PROJECT NO. Stainless Cable 216-1 SHEET 1006 OF

PROJECT NAME DESIGNED BY AF DATE

SUBJECT Fascia Mount CHECKED BY DATE

∴ Allowable Tension load $= 0.65 * \frac{8400\ (lb)}{1.6}$

$T_S = 3,412$ lb

* Check shear strength – Concrete breakout strength in shear

$$V_{cb} = \frac{A_{Vc}}{A_{Vco}} \psi_{ec,V}\, \psi_{ed,V}\, \psi_{c,V}\, \psi_{h,V}\, V_b \qquad (17.5.2.1b)$$

$$A_{Vc} = 1.5 C_{a1} (1.5 * 2 C_{a1} + S_1)$$

$$= 1.5 * 3 (1.5 * 2 * 2.5 + 3.52) = 49.59 \text{ in}^2$$

$A_{Vco} = 4.5 C_{a1}^2$, C_a = edge distance

$$= 4.5 * 2.5^2 = 28.125$$

$\psi_{cV} = 1.0$ Cracked Concrete

$$\psi_{h,V} = \sqrt{1.5 (C_{a1}/h_a)} = 1.5 * \frac{2.5}{3} = 1.25$$

h = member thickness, Assum $h_a = 3$

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Medford Office
836 Mason Way (off Sage Road) • Medford, OR 97501
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PROJECT NO. stainless cable 216-1 SHEET 1007 OF ______

PROJECT NAME ______ DESIGNED BY AF DATE ______

SUBJECT fascia Mount CHECKED BY ______ DATE ______

$$V_b = \left[7\left(\frac{l_e}{d_a}\right)^{0.2}\sqrt{d_a}\right] * \lambda_a \sqrt{f'_c}\,(C_{a1})^{1.5} \quad (17.5.2.2a)$$

$$= \left[7 * \left(\frac{4}{3/8}\right)^{0.2} * \sqrt{3/8}\right] * 1.0 * \sqrt{2500} * 3^{1.5}$$

$$= 1788 \text{ (lb)}$$

$$V_{cb} = \frac{49.59}{78.125} * 1.25 * 1788 = 3940 \text{ lb}$$

Bolts shear strength = 1830 (lb) * 2 = 3600 lb (2 = no. of bolts)

ESR-2427 Table-4 (→ 1830 lb)

Allowable shear strength = $\phi V_N / 1.6$

$$= 0.7 * 1788 \text{(lb)} / 1.6$$

$$= 782.25 \text{ lb}$$

Applied shear load: $\frac{50 \text{ lb/ft} * 5}{782.25} = 0.32 > 0.2$

∴ use shear-tension interaction

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PROJECT NO. Stainless Cable 216-1 SHEET 1008 OF ______

PROJECT NAME ______ DESIGNED BY AF DATE ______

SUBJECT Fascia Mount CHECKED BY ______ DATE ______

Tension load utility: $\frac{\text{Applied Tension}}{\text{Allowable Tension}}$

$= \frac{2044\ (lb)}{3412\ (lb)} = 0.599 > 0.2$

$\therefore \frac{N_{ua}}{\phi N_n} + \frac{V_{ua}}{\phi V_n} \ngtr 1.2$

$0.599 + 0.32 = 0.919 < 1.2$ (OK)

$\therefore$ Use [4 - $\frac{3}{8}$" ϕ Red head ITW or LDT w/ min. 4" Embed. & 3" edge distance.

$f'_c \ngtr 2500$ PSI, Concrete member thickness $\ngtr 3$"]

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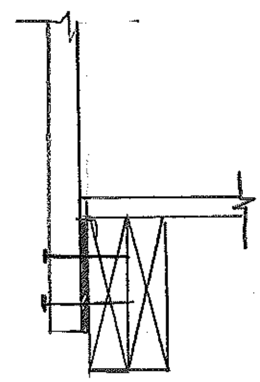
PROJECT NO. Stainless Cable 216-1 SHEET 1009 OF ______

PROJECT NAME ______ DESIGNED BY AF DATE ______

SUBJECT Fascia mount CHECKED BY ______ DATE ______

Fascia mount to wood

wood specific weight = 0.43



Tension load = 2044 lb

∴ Tension / bolt = $\frac{2044^{lb}}{2}$ = 1022 lb

$\frac{3}{8}$" φ lag screw widthdrawl capacity = 243 lb/in (page 1030)

min embed = $\frac{1022^{lb}}{243 (lb/in)}$ = 4.2 in

check shear

Shear load = 250 lb

$\frac{3}{8}$" φ lag screw shear capacity = 150 lb (page 1031)

∴ total shear capacity of 4 screws = 4 * 150 = 600 lb > 250 lb (OK)

use [4 - $\frac{3}{8}$" φ lag screw w/ min. 5".0 Embed]

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PROJECT NO. Stainless Cable 216-1 SHEET 1010 OF ____
PROJECT NAME ____ DESIGNED BY HF DATE ____
SUBJECT Fascia mount CHECKED BY ____ DATE ____

post is directly attached with no bracket

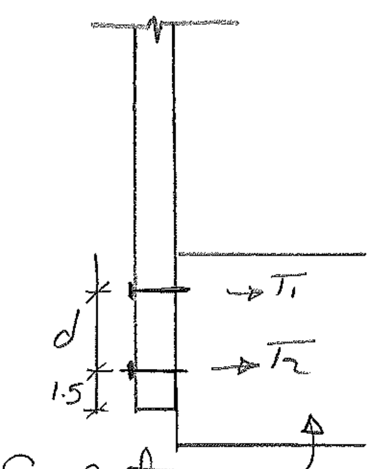
* with Concrete

$\frac{3}{8}$" ϕ redhead ITW

Allowable Tension load / bolt:

$$0.65 * \frac{4200}{1.6} \text{ lb}$$

$$= 1706 \text{ lb}$$



Applied Tension: $\frac{M}{d}$

\# of bolts to resist = 1.0

$$\frac{M}{d} \ngtr 1706 \text{ lb}$$

$$\frac{875 (\text{lb/ft}) * 12}{d} = 1706 \text{ lb}$$

$$d = 6.15$$

∴ Spacing between anchors is 7"

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Tel (541) 850-6300 • FAX (541) 850-6233

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836 Mason Way (off Sage Road) • Medford, OR 97501
Tel (541) 858-8500

www.structure1.com • Email: info@structure1.com

PROJECT NO. Steinless Cable 216-1 SHEET 10 11 OF

PROJECT NAME DESIGNED BY AF DATE

SUBJECT Fascia Mount CHECKED BY DATE

∴ use [2 - 1/2" redhead ITW or LDT
7" apart w/ min 4" Embed.
post to extend 1.5" below bottom anchor
f'c ≮ 2500 psi, Concrete member
thickness ≮ 3"]

Precision Structural Engineering, Inc.
250 Main Street, Suite A • Klamath Falls, OR 97601
Tel (541) 850-6300 • FAX (541) 850-6233

Medford Office
836 Mason Way (off Sage Road) • Medford, OR 97501
Tel (541) 858-8500

www.structure1.com • Email: info@structure1.com

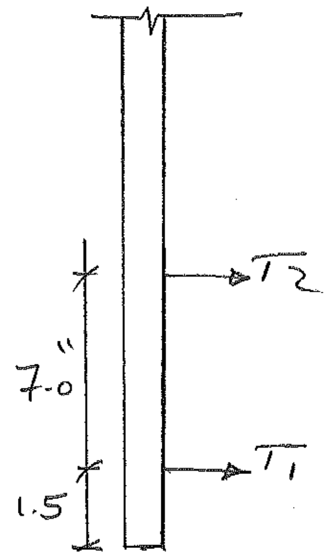
PROJECT NO. Stainless Cable 216-1 SHEET 1012 OF ______

PROJECT NAME ______ DESIGNED BY AF DATE ______

SUBJECT Fascia Mount CHECKED BY ______ DATE ______

* with wood

d = 7" (spacing between lag screws)



M@ bottom of post = 875 lb.ft

Resisting moment = $T_1 * 1.5 + T_2 * 8.5$

$T_1 = T_2 * \frac{1.5}{8.5}$

∴ Resisting moment = $1.5 * \frac{1.5}{8.5} * T_2 + 8.5 T_2$

= $8.76 T_1$ (in. load)

for equilibrium : M = resisting moment

875 (lb.ft) * 12 = $8.76 T_1$

∴ T_1 = 1198 lb

∴ Embed. length = $\frac{T}{\text{withdrawl capacity}}$

= $\frac{1198}{243}$ = 4.9"

∴ use [2 - $\frac{1}{2}$" φ lag screws, 7" apart w/min 6" Emb.]

1013

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DIVISION: 03 00 00—CONCRETE
SECTION: 03 16 00—CONCRETE ANCHORS
DIVISION: 05 00 00—METALS
SECTION: 05 05 19—POST-INSTALLED CONCRETE ANCHORS

REPORT HOLDER:

ITW RED HEAD

700 HIGH GROVE BOULEVARD
GLENDALE HEIGHTS, ILLINOIS 60139

EVALUATION SUBJECT:

ITW RED HEAD CARBON STEEL TRUBOLT+ WEDGE ANCHORS, STAINLESS STEEL TRUBOLT+ WEDGE ANCHORS AND CARBON STEEL OVERHEAD TRUBOLT+ WEDGE ANCHORS FOR CRACKED AND UNCRACKED CONCRETE

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DIVISION: 03 00 00—CONCRETE
Section: 03 16 00—Concrete Anchors

DIVISION: 05 00 00—METALS
Section: 05 05 19—Post-installed Concrete Anchors

REPORT HOLDER:

ITW RED HEAD
700 HIGH GROVE BOULEVARD
GLENDALE HEIGHTS, ILLINOIS 60139
(800) 848-5611
www.itw-redhead.com
techsupport@itwccna.com

ADDITIONAL LISTEE:

ITW BRANDS
955 NATIONAL PARKWAY, SUITE 95500
SCHAUMBURG, ILLINOIS 60173
(877) 489-2726
www.itwbrands.com

EVALUATION SUBJECT:

ITW RED HEAD CARBON STEEL TRUBOLT+ WEDGE ANCHORS, STAINLESS STEEL TRUBOLT+ WEDGE ANCHORS AND CARBON STEEL OVERHEAD TRUBOLT+ WEDGE ANCHORS FOR CRACKED AND UNCRACKED CONCRETE

1.0 EVALUATION SCOPE

Compliance with the following codes:

- 2015, 2012, 2009, and 2006 *International Building Code*® (IBC)
- 2015, 2012, 2009, and 2006 *International Residential Code*® (IRC)
- 2013 *Abu Dhabi International Building Code* (ADIBC)†

†The ADIBC is based on the 2009 IBC. 2009 IBC code sections referenced in this report are the same sections in the ADIBC.

Property evaluated:

Structural

2.0 USES

The carbon steel and stainless steel Trubolt+ Wedge Anchors and $^3/_8$-inch-diameter (9.5 mm) carbon steel OVERHEAD Trubolt+ Wedge Anchor are used to resist static, wind, and seismic tension and shear loads (Seismic Design Categories A thru F) in cracked and uncracked normal-weight and lightweight concrete having a specified compressive strength, f'_c, ranging from 2,500 psi to 8,500 psi (17.2 MPa to 58.6 MPa) [minimum of 24 MPa is required under ADIBC Appendix L, Section 5.1.1].

The carbon steel Trubolt+ Wedge Anchors with diameters of $^3/_8$ inch (9.5 mm), $^1/_2$ inch (12.7 mm) and $^5/_8$-inch (15.9 mm) and the carbon steel OVERHEAD $^3/_8$-inch-diameter (9.5 mm) are used to resist static, wind, and seismic tension and shear loads in cracked and uncracked normal-weight or sand-lightweight concrete over steel deck having a minimum specified compressive strength, f'_c, of 3,000 psi (20.7 MPa) [minimum of 24MPa is required under ADIBC Appendix L, Section 5.1.1].

The Trubolt+ Wedge anchors comply with anchors as described in Section 1901.3 of the 2015 IBC, Section 1909 of the 2012 IBC, and Section 1912 of the 2009 and 2006 IBC. The anchors are alternatives to cast-in-place anchors described in Section 1908 of the 2012 IBC, and Section 1911 of the 2009 and 2006 IBC. The anchors may also be used where an engineered design is submitted in accordance with Section R301.1.3 of the IRC.

3.0 DESCRIPTION

3.1 RED HEAD Carbon Steel Trubolt+ Wedge Anchor:

The RED HEAD Trubolt+ Wedge Anchor is a torque-controlled, wedge-type mechanical expansion anchor, available in $^3/_8$-inch (9.5 mm), $^1/_2$-inch (12.7 mm), $^5/_8$-inch (15.9 mm) and $^3/_4$-inch (19.1 mm) diameters. The Trubolt+ Wedge Anchor consists of a high-strength threaded anchor body, expansion clip, hex nut and washer. The anchor body is cold-formed from low carbon steel materials conforming to AISI 1015 or AISI 1018 with mechanical properties (yield and ultimate strengths) as described in Tables 3 and 4 of this report. The zinc plating on the anchor body complies with ASTM B633 SC1, Type III, with a minimum 0.0002-inch (5 μm) thickness. The expansion clip is fabricated from low carbon steel materials conforming to AISI 1020. The standard hexagonal steel nut conforms to ANSI B18.2.2-65 and the washer conforms to ANSI/ASME B18.22.1 1965 (R1981). The Trubolt+ Wedge anchor body consists of a threaded section throughout the majority of its length and a wedge section at the far end. The expansion clip is formed around the anchor, just above the wedge. The expansion clip consists of a split cylindrical ring with undercutting grooves at the bottom end. During torquing of the anchor, the grooves in the expansion clip are designed to cut into the walls of the concrete hole as the wedge portion of the stud is forced upward against the interior of the clip (U.S. patent nos. 7,744,320 and 7,811,037). The Trubolt+ Wedge anchor is illustrated in Figure 1 of this report.

3.2 RED HEAD Stainless Steel Trubolt+ Wedge Anchor:

The RED HEAD Trubolt+ Wedge Anchor is a torque-controlled, wedge-type mechanical expansion anchor, available in $^1/_2$-inch (12.7 mm) and $^5/_8$-inch (15.9 mm) diameters. The Trubolt+ Wedge Anchor consists of a high-strength threaded anchor body, expansion clip, hex nut and washer. The anchor body is cold-formed from AISI Type 316 stainless steel materials with mechanical properties (yield and ultimate strengths) as described in Tables 5 and 6 of this report. The expansion clip is fabricated from Type 316 stainless steel materials. The Type 316 stainless steel hexagonal steel nut conforms to ANSI B18.2.2-65 and the AISI Type 316 stainless steel washer conforms to ANSI/ASME B18.22.1 1965 (R1981). The Trubolt+ Wedge anchor body consists of a threaded section throughout the majority of its length and a wedge section at the far end. The expansion clip is formed around the anchor, just above the wedge. The expansion clip consists of a split cylindrical ring with undercutting grooves at the bottom end. During torquing of the anchor, the grooves in the expansion clip are designed to cut into the walls of the concrete hole as the wedge portion of the stud is forced upward against the interior of the clip. The Trubolt+ Wedge anchor is illustrated in Figure 1 of this report.

3.3 OVERHEAD Trubolt+ Wedge Anchor:

The OVERHEAD Trubolt+ Wedge Anchor is a torque-controlled, wedge-type mechanical expansion anchor, available in $^3/_8$-inch (9.5 mm) diameter. The OVERHEAD Trubolt+ Wedge Anchor consists of a high-strength threaded anchor body, expansion clip, coupling nut and washer. The anchor body is cold-formed from low carbon steel materials with the mechanical properties (yield and ultimate strengths) as described in Tables 3 and 4 of this report. The zinc plating on the anchor body complies with ASTM B633 SC1, Type III, with a minimum 0.0002 inch (5 μm) thickness. The expansion clip is fabricated from low carbon steel materials. The coupling nut consists of Grade 2 steel with $^3/_8$" -16 threads throughout the length of the nut. The washer complies with ANSI/ASME B18.22.1 1965 (R1981). The OVERHEAD Trubolt+ Wedge anchor body consists of a threaded section throughout the majority of its length and a wedge section at the far end. The expansion clip is formed around the anchor, just above the wedge. The expansion clip consists of a split cylindrical ring with undercutting grooves at the bottom end. During torquing of the anchor (using coupling nut), the grooves in the expansion clip are designed to cut into the walls of the concrete hole as the wedge portion of the anchor body is forced upward against the interior of the clip (U.S. patent nos. 7,744,320 and 7,811,037). The OVERHEAD Trubolt+ Wedge anchor is illustrated in Figure 2 of this report.

3.4 Concrete:

Normal-weight and lightweight concrete must comply with Sections 1903 and 1905 of the IBC.

3.5 Steel Deck Panels:

Steel deck panels must comply with ASTM A653, SS Grade 40 (minimum), and must have a minimum 0.034-inch (0.864 mm) base-metal thickness (No. 20 gage).

4.0 DESIGN AND INSTALLATION

4.1 Strength Design:

4.1.1 General: Design strength of anchors complying with the 2015 IBC, as well as Section R301.1.3 of the 2015 IRC must be determined in accordance with ACI 318-14 Chapter 17 and this report.

Design strength of anchors in accordance with the 2012 IBC, as well as Section R301.1.3 of the 2012 IRC, must be determined in accordance with ACI 318-11 Appendix D and this report.

Design strength of anchors in accordance with the 2009 IBC and Section R301.1.3 of the 2009 IRC must be in accordance with ACI 318-08 Appendix D and this report.

Design strength of anchors in accordance with the 2006 IBC and Section R301.1.3 of the 2006 IRC must be in accordance with ACI 318-05 Appendix D and this report.

Design parameters are based on the 2015 IBC (ACI 318-14) and 2012 IBC (ACI 318-11) unless noted otherwise in Sections 4.1.1 through 4.1.12 of this report.

The strength design of anchors must comply with ACI 318-14 17.3.1 or ACI 318-11 D.4.1, as applicable, except as required in ACI 318-14 17.2.3 or ACI 318-11 D.3.3, as applicable. Strength reduction factors, ϕ, as given in ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable, must be used for load combinations calculated in accordance with Section 1605.2 of the IBC and Section 5.3 of ACI 318-14 or Section 9.2 of ACI 318-11, as applicable. Strength reduction factors, ϕ, as given in ACI 318-11 D.4.4 must be used for load combinations calculated in accordance with ACI 318-11 Appendix C.

The value of f'_c used in calculations must be limited to 8,000 psi (55.2 MPa), maximum, in accordance with ACI 318-14 17.2.7 or ACI 318-11 D.3.7, as applicable. An example calculation in accordance with 2015 and 2012 IBC is provided in Table 7.

4.1.2 Requirements for Static Steel Strength in Tension, N_{sa}: The nominal static steel strength of a single anchor in tension, N_{sa}, calculated in accordance with ACI 318-14 17.4.1.2 or ACI 318-11 Section D.5.1.2, as applicable, is given in Tables 3 or 5 of this report. Strength reduction factors, ϕ, corresponding to ductile steel elements may be used for tension.

4.1.3 Requirements for Static Concrete Breakout Strength in Tension, N_{cb}, N_{cbg}: The nominal concrete breakout strength of a single anchor or a group of anchors in tension, N_{cb} or N_{cbg} respectively, must be calculated in accordance with ACI 318-14 17.4.2 or ACI 318-11 D.5.2, as applicable, with modifications as described in this section. The basic concrete breakout strength of a single anchor in tension, N_b, must be calculated in accordance with ACI 318-14 17.4.2.2 or ACI 318-11 D.5.2.2, as applicable, using the values of h_{ef} and k_{cr} as given in Tables 3 or 5 of this report. The nominal concrete breakout strength in tension in regions where analysis indicates no cracking in accordance with ACI 318-14 17.4.2.6 or ACI 318-11 D.5.2.6, as applicable, must be calculated with $\psi_{c,N}$ = 1.0 and using the value of k_{uncr} as given in Tables 3 or 5 of this report.

For anchors installed in the soffit of sand-lightweight or normal-weight concrete-filled steel deck floor and roof assemblies, as shown in Figure 7, calculation of the concrete breakout strength in accordance with ACI 318-14 17.4.2 or ACI 318-11 D.5.2, as applicable, is not required.

4.1.4 Requirements for Static Pullout Strength in Tension, N_{pn}: The nominal pullout strength of a single anchor in tension in accordance with ACI 318-14 17.4.3 or ACI 318-11 D.5.3, as applicable, in cracked and uncracked concrete, $N_{p,cr}$ or $N_{p,uncr}$, respectively, is given in Tables 3 or 5 of this report. For all design cases, $\psi_{c,P}$ = 1.0. In accordance with ACI 318-14 17.4.3.2 or ACI 318-11 D.5.3.2, as applicable, the nominal pullout strength in tension must be calculated according to Eq-1.

$$N_{p,f'c} = N_{p,cr}\sqrt{\frac{f'_c}{2,500}} \text{ (lb, psi)} \qquad \text{(Eq-1)}$$

$$N_{p,f'c} = N_{p,cr}\sqrt{\frac{f'_c}{17.2}} \text{ (N, MPa)}$$

In regions where analysis indicates no cracking in accordance with ACI 318-14 17.4.3.6 or ACI 318-11 D.5.3.6, as applicable, the nominal pullout strength in tension must be calculated according to Eq-2:

$$N_{p,f'c} = N_{p,uncr}\sqrt{\frac{f'_c}{2,500}} \text{ (lb, psi)} \qquad \text{(Eq-2)}$$

$$N_{p,f'c} = N_{p,uncr}\sqrt{\frac{f'_c}{17.2}} \text{ (N, MPa)}$$

where values for $N_{p,cr}$ or $N_{p,uncr}$ are not provided in Tables 3 or 5 of this report, the pullout strength in tension need not be evaluated.

The nominal pullout strength in tension of the anchors installed in the soffit of sand lightweight or normal-weight concrete on steel deck floor and roof assemblies, as shown in Figure 7 of this report, is given in Table 9. In accordance with ACI 318-14 17.4.3.2 or ACI 318-11 D.5.3.2, as applicable, the nominal pullout strength in cracked concrete must be calculated according to Eq-1, whereby the value of $N_{p,deck,cr}$ must be substituted for $N_{p,cr}$ and the value 3,000 psi or 20.7 MPa must be substituted for 2,500 psi or 17.2 MPa. In regions where analysis indicates no cracking in accordance with ACI 318-14 17.4.3.6 or ACI 318-11 D.5.3.6, as applicable, the nominal pullout strength in tension must be calculated according to Eq-2, whereby the value of $N_{p,deck,uncr}$ must be substituted for $N_{p,uncr}$ and the value 3,000 psi or 20.7 MPa must be substituted for 2,500 psi or 17.2 MPa.

4.1.5 Requirements for Static Steel in Shear, V_{sa}: The values of V_{sa} for a single anchor given in Tables 4 or 6 of this report must be used in lieu of the values of V_{sa} derived by calculation according to ACI 318-14 17.5.1.2 or ACI 318-11 D.6.1.2, as applicable. Strength reduction factors, ϕ, corresponding to ductile steel elements may be used except for the carbon steel $^3/_8$-inch-diameter (9.5 mm) anchors loaded in shear, which have a strength reduction factor corresponding to brittle steel elements.

The shear strength, $V_{sa,deck}$, of anchors installed in the soffit of sand lightweight or normal-weight concrete on steel deck floor and roof assemblies, as shown in Figure 7 of this report, is given in Table 9 of this report.

4.1.6 Requirements for Static Concrete Breakout Strength in Shear, V_{cb} or V_{cbg}: The nominal static concrete breakout strength in shear of a single anchor or a group of anchors, V_{cb} or V_{cbg}, must be calculated in accordance with ACI 318-14 17.5.2 or ACI 318-11 D.6.2, as applicable. The basic concrete breakout strength in shear of a single anchor in cracked concrete, V_b, must be calculated in accordance with ACI 318-14 17.5.2.2 or ACI 318-11 D.6.2.2, as applicable, using the value of d_a, given in Table 2 of this report, and the value l_e, given in Tables 4 or 6, must be taken no greater than h_{ef}. In no cases must l_e exceed $8d_a$.

For anchors installed in the soffit of sand lightweight or normal-weight concrete on steel deck floor and roof assemblies, as shown in Figure 7, calculation of the concrete breakout strength in accordance with ACI 318-14 17.5.2 or ACI 318-11 D.6.2, as applicable, is not required.

4.1.7 Requirements for Static Concrete Pryout Strength of Anchor in Shear, V_{cp} or V_{cpg}: The nominal static concrete pryout strength in shear of a single anchor or groups of anchors, V_{cp} or V_{cpg}, must be calculated in accordance with ACI 318-14 17.5.3 or ACI 318-11 D.6.3, as applicable, modified by using the value of k_{cp} provided in Tables 4 and 6 of this report and the value of N_{cb} or N_{cbg} as calculated in Section 4.1.3 of this report.

For anchors installed in the soffit of sand-lightweight or normal-weight concrete on steel deck floor and roof assemblies, as shown in Figure 7 of this report, calculation of the concrete pryout strength in accordance with ACI 318-14 17.5.3 or ACI 318-11 D.6.3 is not required.

4.1.8 Requirements for Minimum Member Thickness, Minimum Anchor Spacing and Minimum Edge Distance: Values of s_{min} and c_{min} as given in Table 2 of this report must be used in lieu of ACI 318-14 17.7.1 and 17.7.3 or ACI 318-11 D.8.1 and D.8.3, respectively, as applicable. Minimum member thicknesses, h_{min}, as given in Tables 2 through 6 of this report, must be used in lieu of ACI 318-14 17.7.5 or ACI 318-11 D.8.5, as applicable.

For anchors installed in the soffit of sand-lightweight or normal-weight concrete on steel deck floor and roof assemblies, the anchors must be installed in accordance with Figure 7 of this report and the minimum anchor spacing along the flute must be the greater of $3h_{ef}$ or 1.5 times the flute width.

4.1.9 Requirements for Critical Edge Distance and Splitting: In applications where $c < c_{ac}$ and supplemental reinforcement to control splitting of the concrete is not present, the concrete breakout strength in tension for uncracked concrete, calculated according to ACI 318-14 17.4.2 or ACI 318-11 D.5.2, as applicable, must be further multiplied by the factor $\Psi_{cp,N}$ given by Eq-3:

$$\Psi_{cp,N} = c / c_{ac} \qquad \text{(Eq-3)}$$

whereby the factor $\Psi_{cp,N}$ need not be taken as less than $1.5h_{ef} / c_{ac}$. For all other cases $\Psi_{cp,N}$ = 1.0. Values for the critical edge distance c_{ac} must be taken from Tables 3 or 5 of this report.

4.1.10 Requirements for Seismic Design:

4.1.10.1 General: For load combinations including earthquake, the design must be performed according to ACI 318-14 17.2.3 or ACI 318-11 D.3.3, as applicable. Modifications to ACI 318-14 17.2.3 shall be applied under Section 1905.1.8 of the 2015 IBC. For the 2012 IBC, Section 1905.1.9 is omitted. Modifications to ACI 318 (-08, -05) D.3.3 must be applied under Section 1908.1.9 of the 2009 IBC or Section 1908.1.16 of the 2006 IBC, as applicable.

The carbon steel $^1/_2$-inch- $^5/_8$-inch- and $^3/_4$-inch-diameter (12.7, 15.9 and 19.1 mm), stainless steel $^1/_2$-inch (12.7 mm) and $^5/_8$-inch (15.9 mm) anchors loaded in tension and shear, along with the $^3/_8$-inch-diameter (9.5 mm) anchor loaded in tension only, comply with ACI 318-14 2.3 or ACI 318-11 D.1, as applicable, as ductile steel elements and must be designed in accordance with ACI 318-14, 17.2.3.4, 17.2.3.5, 17.2.3.6 or 17.2.3.7; ACI 318-11 D.3.3.4, D.3.3.5, D.3.3.6, or D.3.3.7; ACI 318-08 D.3.3.4, D.3.3.5, or D.3.3.6; or ACI 318-05 D.3.3.4 or D.3.3.5, as applicable.

The carbon steel $^3/_8$-inch-diameter (9.5 mm) anchor loaded in shear must be designed in accordance with ACI 318-14 17.2.3.5.3, ACI 318-11 D.3.3.5.3, ACI 318-08 D.3.3.5 or D.3.3.6, or ACI 318-05 D.3.3.6 as brittle steel elements, as applicable.

4.1.10.2 Seismic Tension: The nominal steel strength and nominal concrete breakout strength for anchors in tension must be calculated according to ACI 318-14 17.4.1 and 17.4.2 or ACI 318-11 D.5.1 and D.5.2, respectively, as applicable, as described in Sections 4.1.2 and 4.1.3 of this report. In accordance with ACI 318-14 17.4.3.2 or ACI 318-11 D.5.3.2, as applicable, the value for nominal pullout strength tension for seismic loads, N_{eq} or $N_{p,deck,cr}$, given in Table 3, 5 or 9 of this report, must be used in lieu of N_p. The values of N_{eq} must be adjusted for the concrete strength in accordance with Eq-4:

$$N_{eq,f'c} = N_{eq}\sqrt{\frac{f'_c}{2{,}500}} \text{ (lb, psi)} \qquad \text{(Eq-4)}$$

$$N_{eq,f'c} = N_{eq}\sqrt{\frac{f'_c}{17.2}} \text{ (N, MPa)}$$

The value of $N_{p,deck,cr}$ must be calculated according to Eq-4, whereby the value 3,000 psi or 20.7 MPa must be substituted for 2,500 psi or 17.2 MPa.

If no values for N_{eq} are given in Tables 3 or 5, the static design strength values govern. Section 4.1.4 provides additional requirements.

4.1.10.3 Seismic Shear: The nominal concrete breakout strength and pryout strength for anchors in shear must be calculated according to ACI 318-14 17.5.2 and 17.5.3 or ACI 318-11 D.6.2 and D.6.3, respectively, as applicable, as described in Sections 4.1.6 and 4.1.7 of this report. In accordance with ACI 318-14 17.5.1.2 or ACI 318-11 D.6.1.2, as applicable, the value for nominal steel strength in shear for seismic loads, V_{eq}, or $V_{sa,deck}$, given in Tables 4, 6 or 9 of this report, must be used in lieu of V_{sa}.

4.1.11 Interaction of Tensile and Shear Forces: For designs that include combined tension and shear, the interaction of tension and shear loads must be calculated in accordance with ACI 318-14 17.6 or ACI 318-11 D.7, as applicable.

4.1.12 Lightweight Concrete: For the use of anchors in lightweight concrete, the modification factor λ_a equal to 0.8λ is applied to all values of $\sqrt{f'_c}$ affecting N_n and V_n.

For ACI 318-14 (2015 IBC), ACI 318-11 (2012 IBC) and ACI 318-08 (2009 IBC), λ shall be determined in accordance with the corresponding version of ACI 318.

For ACI 318-05 (2006 IBC), λ shall be taken as 0.75 for all lightweight concrete and 0.85 for sand-lightweight concrete. Linear interpolation shall be permitted if partial sand replacement is used. In addition, the pullout strengths $N_{p,cr}$, $N_{p,uncr}$, and N_{eq} shall be multiplied by the modification factor, λ_a, as applicable.

For anchors installed in the soffit of sand-lightweight concrete-filled steel deck and floor and roof assemblies, further reduction of the pullout values provided in this report is not required.

4.2 Allowable Stress Design (ASD):

4.2.1 General: For anchors designed using load combinations in accordance with IBC Section 1605.3, allowable loads must be established using Eq-5 and Eq-6:

$$T_{allowable,ASD} = \phi N_n / \alpha \qquad \text{(Eq-5)}$$

and

$$V_{allowable,ASD} = \phi V_n / \alpha \qquad \text{(Eq-6)}$$

where

$T_{allowable,ASD}$ = Allowable tension load (lbf or kN).

$V_{allowable,ASD}$ = Allowable shear load (lbf or kN).

ϕN_n = Lowest design strength of an anchor or anchor group in tension as determined in accordance with ACI 318-14 Chapter 17 and 2015 IBC Section 1905.1.8, ACI 318-11 Appendix D, ACI 318-08 Appendix D and 2009 IBC Section 1908.1.9 and ACI 318-05 Appendix D and 2006 IBC Section 1908.1.16, and Section 4.1 of this report, as applicable.

ϕV_n = Lowest design strength of an anchor or anchor group in shear as determined in accordance with ACI 318-14 Chapter 17 and 2015 IBC Section 1905.1.8, ACI 318-11 Appendix D, ACI 318-08 Appendix D and 2009 IBC Section 1908.1.9, ACI 318-05 Appendix D and 2006 IBC Section 1908.1.16, and Section 4.1 of this report, as applicable.

α = Conversion factor calculated as a weighted average of the load factors for the controlling load combination. In addition, α must include all applicable factors to account for nonductile failure modes and required over-strength.

An example of allowable stress design values for illustrative purposes is shown in Table 7 of this report.

4.2.2 Interaction of Tensile and Shear Forces: In lieu of ACI 318-14 17.6.1, 17.6.2 and 17.6.3 or ACI 318 (-11, -08, -05) D.7.1, D.7.2 and D.7.3, interaction must be calculated as follows:

For shear loads $V \leq 0.2\ V_{allowable,\ ASD}$, the full allowable load in tension, $T_{allowable,\ ASD}$, may be taken.

For tension loads $T \leq 0.2\ T_{allowable,\ ASD}$, the full allowable load in shear, $V_{allowable,\ ASD}$, may be taken.

For all other cases, Eq-7 applies:

$$T/T_{allowable,\ ASD} + V/V_{allowable,\ ASD} \leq 1.2 \qquad \text{(Eq-7)}$$

For the OVERHEAD Trubolt+ Wedge Anchor, the influence of bending on the tension capacity when loaded in shear must be considered.

4.3 Installation:

Installation parameters are provided in Tables 2 and 8 and Figures 4, 5, and 6 of this report. Anchor locations must comply with this report and the plans and specifications approved by the code official. The Trubolt+ Wedge Anchors must be installed according to ITW's published instructions and this report. Holes must be predrilled in concrete with a compressive strength from 2,500 to 8,500 psi (17.2 to 58.6 MPa) [minimum of 24 MPa is required under ADIBC Appendix L, Section 5.1.1] at time of installation, using carbide-tipped masonry drill bits manufactured within the range of the maximum and minimum drill tip dimensions of ANSI Standard B212.15-1994. The nominal drill bit diameter must be equal to that of the anchor size. The minimum drilled hole depth, h_o, must comply with Table 2 of this report. Embedment, spacing, edge distance, and minimum concrete thickness must comply with Table 2. The predrilled holes must be cleaned to remove loose particles, using pressurized air or a vacuum. For the RED HEAD Trubolt+ Wedge Anchor, the hex nut and washer must be assembled on the end of the anchor, leaving the nut flush with the end of the anchor. For the OVERHEAD Trubolt+ Wedge Anchor, the coupling nut and washer must be assembled on the end of the anchor to obtain at least 1/2 inch (12.7 mm) thread engagement on the anchor). The anchors must be hammered into the predrilled hole to the required embedment depth in concrete. Where a fixture is installed, the anchors must be hammered through the fixture into the predrilled hole to the required embedment depth into the concrete. The nut must be tightened against the washer until the specified torque values listed in Table 2 are achieved.

For installation in the soffit of sand-lightweight or normal-weight concrete on steel deck floor and roof assemblies, the hole diameter in the steel deck must not exceed the diameter of the hole in the concrete by more than 1/8 inch (3.2 mm) and concrete must have a minimum compressive strength of 3,000 psi (20.7 MPa) [minimum of 24 MPa is required under ADIBC Appendix L, Section 5.1.1] at time of installation. For member thickness, edge distance, spacing restrictions, and installations torque values for installation into the soffit of sand lightweight or normal-weight concrete on steel deck floor and roof assemblies, see Figure 7, Table 8, and Section 4.1.8 of this report.

4.4 Special Inspection:

Periodic special inspection is required, in accordance with Section 1705.1.1 and Table 1705.3 of the 2015 IBC and 2012 IBC; Section 1704.15 and Table 1704.4 of the 2009 IBC; or Section 1704.13 of the 2006 IBC, as applicable. The special inspector must make periodic inspections during anchor installation to verify anchor type, anchor dimensions, concrete type, concrete compressive strength, drill bit type, hole dimensions, hole cleaning procedures, edge distance, anchor spacing, concrete member thickness, anchor embedment, tightening torque, and adherence to the manufacturer's published installation instructions. The special inspector must be present as often as required in accordance with the statement of special inspection. Under the IBC, additional requirements as set forth in Sections 1705, 1706, and 1707 must be observed, where applicable.

5.0 CONDITIONS OF USE

The Trubolt+ Wedge Anchors described in this report comply with, or are suitable alternatives to what is specified in, those codes listed in Section 1.0 of this report, subject to the following conditions.

5.1 The anchors must be installed in accordance with ITW's published instructions and this report. In case of conflicts, this report governs.

5.2 Anchor sizes, dimensions, and installation parameters are as set forth in this report.

5.3 The anchors are limited to installation in cracked and uncracked, normal-weight or lightweight concrete having a specified compressive strength, f'_c, of 2,500 psi to 8,500 psi (17.2 MPa to 58.6 MPa) [minimum of 24 MPa is required under ADIBC Appendix L, Section 5.1.1]. The anchors may also be installed in cracked and uncracked normal-weight or sand-lightweight concrete over profile steel deck having a minimum specified compressive strength, f'_c, of 3,000 psi (20.7 MPa) [minimum of 24 MPa is required under ADIBC Appendix L, Section 5.1.1].

5.4 The values of f'_c used for calculation purposes must not exceed 8,000 psi (55.0 MPa).

5.5 Strength design values must be established in accordance with Section 4.1 of this report.

5.6 Allowable design values must be established in accordance with Section 4.2 of this report.

5.7 Anchor spacing, edge distance, and minimum member thickness must comply with Tables 2 and 8 and Figures 4, 5, and 6 of this report.

5.8 Prior to installation, calculations and details justifying that the applied loads comply with this report must be submitted to the code official for approval. The calculations and details must be prepared by a registered design professional where required by the statutes of the jurisdiction in which the project is to be constructed.

5.9 Since an ICC-ES acceptance criteria for evaluating data to determine the performance of expansion anchors subjected to fatigue or shock loading is unavailable at this time, the use of these anchors under such conditions is beyond the scope of this report.

5.10 Anchors may be installed in regions of concrete where cracking has occurred or where analysis indicates cracking may occur ($f_t > f_r$), subject to the conditions of this report.

5.11 Anchors may be used to resist short-term loading due to wind or seismic forces, subject to the conditions of this report.

5.12 Where not otherwise prohibited in the code, Trubolt+ Wedge Anchors are permitted for use with fire-resistance-rated construction provided that at least one of the following conditions is fulfilled:

- Anchors are used to resist wind or seismic forces only.
- Anchors that support a fire-resistance-rated envelope or a fire-resistance-rated membrane are protected by approved fire-resistance-rated materials, or have been evaluated for resistance to fire exposure in accordance with recognized standards.
- Anchors are used to support nonstructural elements.

5.13 Use of the zinc plated, carbon steel anchors is limited to dry, interior locations.

5.14 Special inspections are provided in accordance with Section 4.4 of this report.

5.15 The anchors are manufactured in the USA; under a quality-control program with inspections by ICC-ES.

6.0 EVIDENCE SUBMITTED

Data complying with the ICC-ES Acceptance Criteria for Mechanical Anchors in Concrete Elements (AC193), dated October 2015, for use in cracked and uncracked concrete, including optional tests for seismic tension and shear; profile steel deck soffit tests; and quality control documentation.

7.0 IDENTIFICATION

The anchors are identified by their dimensional characteristics and the anchor size, and by a length identification marking stamped on the anchor, as indicated in Table 1 of this report. The anchors have the length identification marking underlined on the anchor head, as illustrated in Figure 3 of this report, and this is visible after installation for verification. Packages are identified with the anchor name, material (carbon or stainless) type and size; the manufacturer's name (ITW Red Head, ITW Brands, or ITW Buildex) and address; and the evaluation report number (ESR-2427).

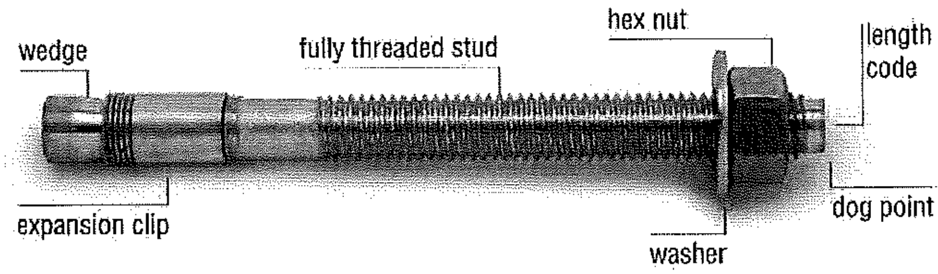


FIGURE 1—ITW RED HEAD TRUBOLT+ WEDGE ANCHOR
(Carbon and Stainless Steel)

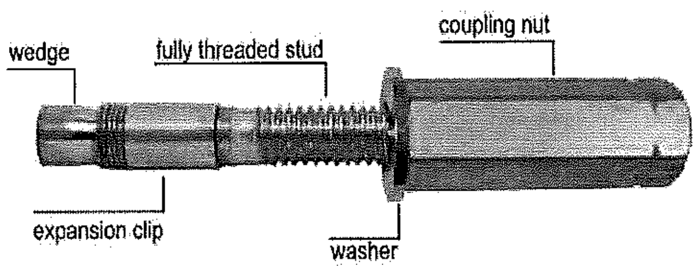


FIGURE 2—OVERHEAD TRUBOLT+ WEDGE ANCHOR

TABLE 1—LENGTH IDENTIFICATION MARKINGS[1]

LENGTH (inches)	ID MARKING ON ANCHOR HEAD												
	C	D	E	F	G	H	I	J	K	L	M	N	O
From	$2^1/_2$	3	$3^1/_2$	4	$4^1/_2$	5	$5^1/_2$	6	$6^1/_2$	7	$7^1/_2$	8	$8^1/_2$
Up to, but not including	3	$3^1/_2$	4	$4^1/_2$	5	$5^1/_2$	6	$6^1/_2$	7	$7^1/_2$	8	$8^1/_2$	9

For **SI:** 1 inch = 25.4 mm.

[1]Figure 3 shows a typical marking.

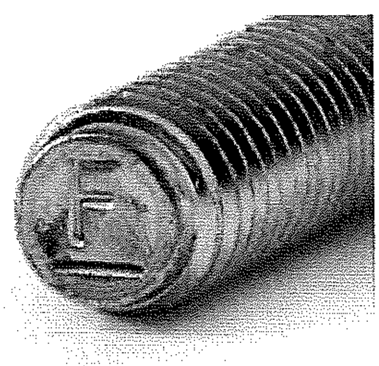


FIGURE 3—TRUBOLT+ WEDGE ANCHOR LENGTH IDENTIFICATION MARKING

TABLE 2—ITW RED HEAD TRUBOLT+ WEDGE ANCHOR AND OVERHEAD TRUBOLT+ WEDGE ANCHOR INSTALLATION INFORMATION (CARBON STEEL AND STAINLESS STEEL)[1]

PARAMETER	NOTATION	UNITS	NOMINAL ANCHOR DIAMETER (inch)									
			3/8		1/2				5/8		3/4	
Anchor outer diameter	$d_a[d_o]$[3]	inches	0.361		0.5				0.615		0.7482	
Nominal carbide bit diameter	d_{bit}	inches	3/8		1/2				5/8		3/4	
Effective embedment depth	h_{ef}	inches	1 5/8		2		3 1/4		2 3/4	4 1/4	3 3/4	
Minimum anchor embedment depth	h_{nom}	inches	2		2 1/2		3 3/4		3 1/4	4 3/4	4 3/8	
Minimum hole depth[1]	h_o	inches	2 1/4		2 3/4		4		3 1/2	5	4 5/8	
Minimum concrete member thickness[1]	h_{min}	inches	4	5	4	6	6	8	6	6 1/4	7	8
Critical edge distance[1]	c_{ac}	In.	5	3	6	6	7 1/2	6	7 1/2	6½	12	10
Minimum anchor spacing[1]	s_{min}	In.	3 1/2	2 1/2	6	5 3/4	4	5 3/4	8	6	6	6
Minimum edge distance[2]	c_{min}	In.	3		6				7 1/2	5	7 1/2	7 1/2
Minimum overall anchor length	l_{anchor}	inches	2 1/2		3 3/4		4 1/2		4 1/4	6	5 1/2	
Installation torque	T_{inst}	ft-lb	30		45				90		110	
Minimum diameter of hole in fastened part	d_h	inches	1/2		5/8				3/4		7/8	

For **SI:** 1 inch = 25.4 mm, 1 ft-lb = 1.356 N-m.

[1]Stainless steel anchors are available in 1/2-inch and 5/8-inch-diameters. The OVERHEAD version is available in a carbon steel 3/8-inch-diameter.
[2]For installation of the carbon steel anchors in the soffit of concrete on steel deck floor or roof assemblies, see Figure 7. Anchors in the lower and in the upper flute may be installed with a maximum 1-inch offset in either direction from the center of the flute. In addition, anchors must have an axial spacing along the flute equal to the greater of $3h_{ef}$ or 1.5 times the flute width.
[3]The notation in brackets is for the 2006 IBC.

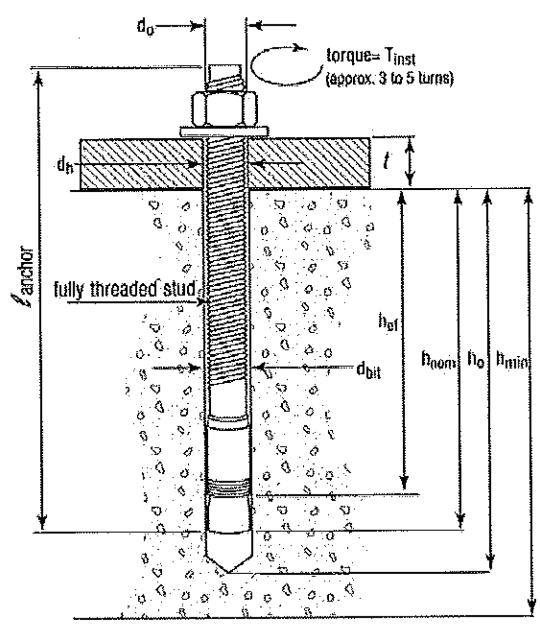


FIGURE 4—ITW RED HEAD TRUBOLT+ WEDGE ANCHOR (INSTALLED)

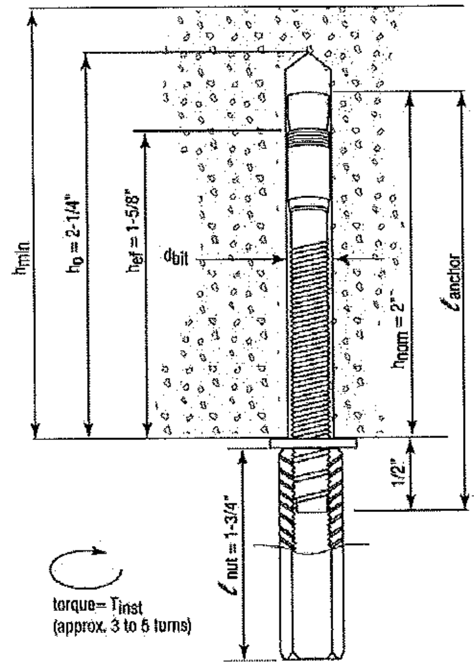
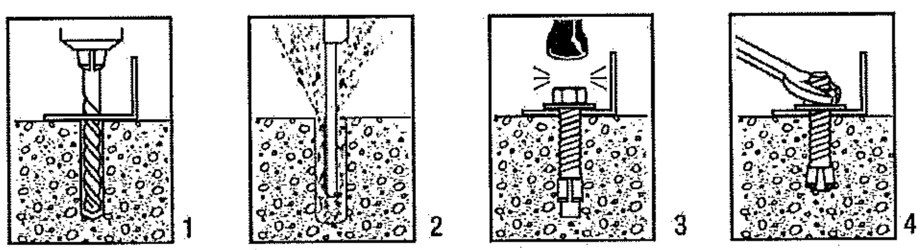


FIGURE 5—OVERHEAD TRUBOLT+ WEDGE ANCHOR (INSTALLED), $^3/_8$ INCH NOMINAL ANCHOR DIAMETER (d_o)



1. Select a carbide drill bit with a diameter equal to the anchor diameter. Drill hole 1/4" deeper than anchor embedment.
2. Clean hole with pressurized air or vacuum to remove any excess dust/debris.
3. Using the washer and nut provided, assemble the anchor, leaving nut one half turn from the end of anchor to protect threads. Drive anchor through fixture to be fastened until washer is flush to surface of fixture.
4. Expand anchor by tightening nut to the specified setting torque (approx 3-5 turns).

FIGURE 6—INSTALLATION INSTRUCTIONS

TABLE 3—ITW RED HEAD CARBON STEEL TRUBOLT+ WEDGE ANCHOR AND OVERHEAD TRUBOLT+ WEDGE ANCHOR TENSION DESIGN INFORMATION[1,2,3,9]

CHARACTERISTIC	SYMBOL	UNITS	NOMINAL ANCHOR DIAMETER (inch)[6]									
			3/8		1/2				5/8		3/4	
Anchor category	1, 2 or 3	—	1		1				1		1	
Minimum effective embedment depth	h_{ef}	in.	1 5/8		2		3 1/4		2 3/4	4 1/4	3 3/4	
Minimum concrete member thickness	h_{min}	in.	4	5	4	6	6	8	6	6 1/4	7	8
Critical edge distance	c_{ac}	in.	5	3	6	6	7 1/2	6	7 1/2	6 1/2	12	10
Data for Steel Strengths – Tension												
Minimum specified yield strength	f_y	psi	60,000		55,000				55,000		55,000	
Minimum specified ultimate strength	f_{uta}	psi	75,000		75,000				75,000		75,000	
Effective tensile stress area (neck)	$A_{se,N}$ $[A_{se}]^8$	in²	0.056		0.119				0.183		0.266	
Steel strength in tension	N_{sa}	lbf	4,200		8,925				13,725		19,950	
Strength reduction factor ϕ for tension, steel failure modes[4]	ϕ	—	0.75		0.75				0.75		0.75	
Data for Concrete Breakout Strengths in Tension												
Effectiveness factor - uncracked concrete	k_{uncr}	—	24		24				24		24	
Effectiveness factor - cracked concrete	k_{cr}	—	17		17				17		17	
Modification factor for cracked and uncracked concrete[5]	$\Psi_{c,N}$	—	1.0		1.0				1.0		1.0	
Strength reduction factor ϕ for tension, concrete failure modes, Condition B[4]	ϕ	—	0.65		0.65				0.65		0.65	
Data for Pullout Strengths												
Pullout strength, uncracked concrete	$N_{p,uncr}$	lbf	See Footnote 7		See Footnote 7		6,540		5,430	8,900	See Footnote 7	
Pullout strength, cracked concrete	$N_{p,cr}$	lbf	See Footnote 7		See Footnote 7				See Footnote 7		See Footnote 7	
Pullout strength for seismic loads	N_{eq}	lbf	See Footnote 7		See Footnote 7				See Footnote 7	6,715	See Footnote 7	
Strength reduction factor ϕ for tension, pullout failure modes, Condition B[4]	ϕ	—	See Footnote 7		0.65				0.65		See Footnote 7	
Additional Anchor Data												
Axial stiffness in service load range in uncracked concrete	β_{uncr}	lbf /in	100,000		250,000				250,000		250,000	
Axial stiffness in service load range in cracked concrete	β_{cr}	lbf /in	40,000		20,000				20,000		20,000	

For **SI:** 1 inch = 25.4 mm, 1 in² = 645.16 mm², 1 lbf = 4.45 N, 1 psi = 0.006895 MPa, 1 lbf · 10²/in = 17,500 N/m.

[1]The data in this table is intended to be used with the design provisions of ACI 318-14 Chapter 17 or ACI 318-11 Appendix D, as applicable; for anchors resisting seismic load combinations, the additional requirements of ACI 318-14 17.2.3 or ACI 318-11 D.3.3, as applicable, shall apply.
[2]Installation must comply with the manufacturers printed installation instructions and details.
[3]The 3/8-, 1/2-, 5/8-, and 3/4-inch diameter Trubolt + Wedge Anchors are ductile steel elements under tension loading as defined by ACI 318-14 2.3 or ACI 318-11 D.1, as applicable.
[4]All values of ϕ apply to the load combinations of IBC Section 1605.2, ACI 318-14 Section 5.3 or ACI 318-11 Section 9.2, as applicable. If the load combinations of ACI 318-11 Appendix C are used, then the appropriate value of ϕ must be determined in accordance with ACI 318-11 D.4.4. For installations where reinforcement that complies with ACI 318-14 Chapter 17 or ACI 318-11 Appendix D, as applicable, requirements for Condition A is present, the appropriate ϕ factor must be determined in accordance with ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable.
[5]For all design cases $\Psi_{c,N}$ = 1.0. The appropriate effectiveness factor for cracked concrete (k_{cr}) or uncracked concrete (k_{uncr}) must be used.
[6]The actual diameter for the 3/8-inch diameter anchor is 0.361 inch, for the 5/8-inch diameter anchor is 0.615-inch, and for the 3/4-inch diameter anchor is 0.7482-inch.
[7]Anchor pullout strength does not control anchor design. Determine steel and concrete capacities only.
[8]The notation in brackets is for the 2006 IBC.
[9]The OVERHEAD version is available in a carbon steel 3/8-inch-diameter only.

TABLE 4—ITW RED HEAD CARBON STEEL TRUBOLT+ WEDGE ANCHOR AND OVERHEAD TRUBOLT+ WEDGE ANCHOR SHEAR DESIGN INFORMATION[1,2,3,8]

CHARACTERISTIC	SYMBOL	UNITS	NOMINAL ANCHOR DIAMETER (inch)[5]									
			3/8		1/2				5/8		3/4	
Anchor category	1, 2 or 3	—	1		1				1		1	
Minimum effective embedment depth	h_{ef}	In.	1 5/8		2		3 1/4		2 3/4	4 1/4	3 3/4	
Minimum concrete member thickness	h_{min}	In.	4	5	4	6	6	8	6	6 1/4	7	8
Critical edge distance	c_{ac}	In.	5	3	6	6	7 1/2	6	7 1/2	6 1/2	12	10
Data for Steel Strengths – Shear												
Minimum specified yield strength	f_y	psi	60,000		55,000				55,000		55,000	
Minimum specified ultimate strength	f_{uta}	psi	75,000		75,000				75,000		75,000	
Effective shear stress area (thread)	$A_{se,V}$ $[A_{se}]^7$	in^2	0.075		0.142				0.217		0.332	
Steel strength in shear, uncracked or cracked concrete[6]	V_{sa}	lbf	1,830		5,175				8,955		14,970	
Steel strength in shear - seismic loads	V_{eq}	lbf	1,545		5,175				8,955		11,775	
Strength reduction factor ϕ for shear, steel failure modes[4]	ϕ	—	0.60		0.65				0.65		0.65	
Data for Concrete Breakout and Concrete Pryout Strengths – Shear												
Coefficient for pryout strength	k_{cp}	—	1.0		1.0		2.0		2.0		2.0	
Load-bearing length of anchor	l_e	in	1 5/8		2		3 1/4		2 3/4	4 1/4	3 3/4	
Strength reduction factor ϕ for shear, concrete failure modes, Condition B[4]	ϕ	—	0.70		0.70				0.70		0.70	

For **SI:** 1 inch = 25.4 mm, 1 in^2 = 645.16 mm^2, 1 lbf = 4.45 N, 1 psi = 0.006895 MPa, 1 lbf · 10^2/in = 17,500 N/m.

[1]The data in this table is intended to be used with the design provisions of ACI 318-14 Chapter 17 or ACI 318-11 Appendix D, as applicable; for anchors resisting seismic load combinations, the additional requirements of ACI 318-14 17.2.3 or ACI 318-11 D.3.3, as applicable, shall apply.
[2]Installation must comply with the manufacturers printed installation instructions and details.
[3]The 1/2-, 5/8-, and 3/4-inch diameter Trubolt + Wedge Anchors are ductile steel elements under shear loading as defined by ACI 318-14 2.3 or ACI 318-11 D.1. The 3/8" diameter Trubolt + is considered brittle under shear loading.
[4]All values of ϕ apply to the load combinations of IBC Section 1605.2, ACI 318-14 5.3 or ACI 318-11 Section 9.2, as applicable. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of ϕ must be determined in accordance with ACI 318-11 D.4.4. For installations where reinforcement that complies with ACI 318-14 Chapter 17 or ACI 318-11 Appendix D requirements for Condition A is present, the appropriate ϕ factor must be determined in accordance with ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable.
[5]The actual diameter for the 3/8" diameter anchor is 0.361-inch, for the 5/8" diameter anchor is 0.615-inch, and for the 3/4" diameter anchor is 0.7482".
[6]Steel strength in shear values are based on test results per ACI 355.2, Section 9.4 and must be used for design.
[7]The notation in brackets is for the 2006 IBC.
[8]The OVERHEAD version is available in a carbon steel 3/8-inch-diameter only.

TABLE 5—ITW RED HEAD STAINLESS STEEL TRUBOLT+ WEDGE ANCHOR TENSION DESIGN INFORMATION[1,2,3]

CHARACTERISTIC	SYMBOL	UNITS	NOMINAL ANCHOR DIAMETER (inch)[6]					
			$^1/_2$				$^5/_8$	
Anchor category	1, 2 or 3	—	1				1	
Minimum effective embedment depth	h_{ef}	In.	2		$3^1/_4$		$2^3/_4$	$4^1/_4$
Minimum concrete member thickness	h_{min}	In.	4	6	6	8	6	$6^1/_4$
Critical edge distance	c_{ac}	In.	6	6	$7^1/_2$	6	$7^1/_2$	$6^1/_2$
Data for Steel Strengths – Tension								
Minimum specified yield strength	f_y	psi	65,000				65,000	
Minimum specified ultimate strength	f_{uta}	psi	100,000				100,000	
Effective tensile stress area (neck)	$A_{se,N}$ $[A_{se}]^8$	in^2	0.119				0.183	
Steel strength in tension	N_{sa}	lbf	11,900				18,300	
Strength reduction factor ϕ for tension, steel failure modes[4]	ϕ	—	0.75				0.75	
Data for Concrete Breakout Strengths in Tension								
Effectiveness factor - uncracked concrete	k_{uncr}	—	24				24	
Effectiveness factor - cracked concrete	k_{cr}	—	17				17	
Modification factor for cracked and uncracked concrete[5]	$\Psi_{c,N}$	—	1.0				1.0	
Strength reduction factor ϕ for tension, concrete failure modes, Condition B[4]	ϕ	—	0.65				0.65	
Data for Pullout Strengths								
Pullout strength, uncracked concrete	$N_{p,uncr}$	lbf	See Footnote 7		6,540		5,430	8,900
Pullout strength, cracked concrete	$N_{p,cr}$	lbf	See Footnote 7				See Footnote 7	
Pullout strength for seismic loads	N_{eq}	lbf	2,345		See Footnote 7		See Footnote 7	
Strength reduction factor ϕ for tension, pullout failure modes, Condition B[4]	ϕ	—	0.65				0.65	
Additional Anchor Data								
Axial stiffness in service load range in uncracked concrete	β_{uncr}	lbf /in	250,000				250,000	
Axial stiffness in service load range in cracked concrete	β_{cr}	lbf /in	20,000				20,000	

For **SI:** 1 inch = 25.4 mm, 1 in^2 = 645.16 mm^2, 1 lbf = 4.45 N, 1 psi = 0.006895 MPa, 1 lbf · 10^2/in = 17,500 N/m.

[1]The data in this table is intended to be used with the design provisions of ACI 318-14 Chapter 17 or ACI 318-11 Appendix D, as applicable; for anchors resisting seismic load combinations, the additional requirements of ACI 318-14 17.2.3 or ACI 318-11 D.3.3, as applicable, shall apply.
[2]Installation must comply with the manufacturers printed installation instructions and details.
[3]The $^1/_2$- and $^5/_8$-,inch diameter Trubolt + Wedge Anchors are ductile steel elements as defined by ACI 318-14 2.3 or ACI 318-11 D.1, as applicable.
[4]All values of ϕ apply to the load combinations of IBC Section 1605.2, ACI 318-14 Section 5.3 or ACI 318-11 Section 9.2, as applicable. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of ϕ must be determined in accordance with ACI 318-11 D.4.4. For installations where reinforcement that complies with ACI 318-14 Chapter 17 or ACI 318-11 Appendix D, as applicable, requirements for Condition A is present, the appropriate ϕ factor must be determined in accordance with ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable.
[5]For all design cases $\Psi_{c,N}$ = 1.0. The appropriate effectiveness factor for cracked concrete (k_{cr}) or uncracked concrete (k_{uncr}) must be used.
[6]The actual diameter for the $^5/_8$-inch diameter anchor is 0.615-inch.
[7]Anchor pullout strength does not control anchor design. Determine steel and concrete capacities only.
[8]The notation in brackets is for the 2006 IBC.

TABLE 6—ITW RED HEAD STAINLESS STEEL TRUBOLT+ WEDGE ANCHOR SHEAR DESIGN INFORMATION[1,2,3]

CHARACTERISTIC	SYMBOL	UNITS	NOMINAL ANCHOR DIAMETER (inch)[5]					
			1/2				5/8	
Anchor category	1, 2 or 3	—	1				1	
Minimum effective embedment depth	h_{ef}	in.	2		3¼		2¾	4¼
Minimum concrete member thickness	h_{min}	in.	4	6	6	8	6	6¼
Critical edge distance	c_{ac}	in.	6	6	7½	6	7½	6½
Data for Steel Strengths - Shear								
Minimum specified yield strength	f_y	psi	65,000				65,000	
Minimum specified ultimate strength	f_{uta}	psi	100,000				100,000	
Effective shear stress area (thread)	$A_{se,V}$ $[A_{se}]^7$	in²	0.142				0.217	
Steel strength in shear, uncracked or cracked concrete[6]	V_{sa}	lbf	7,265				10,215	
Steel strength in shear - seismic loads	V_{eq}	lbf	5,805				8,105	
Strength reduction factor ϕ for shear, steel failure modes[4]	ϕ	—	0.65				0.65	
Data for Concrete Breakout and Concrete Pryout Strengths - Shear								
Coefficient for pryout strength	k_{cp}	—	1.0		2.0		2.0	
Load-bearing length of anchor	l_e	in	2		3¼		2¾	4¼
Strength reduction factor ϕ for shear, concrete failure modes, Condition B[4]	ϕ	—	0.70				0.70	

For **SI:** 1 inch = 25.4 mm, 1 in² = 645.16 mm², 1 lbf = 4.45 N, 1 psi = 0.006895 MPa, 1 lbf · 10²/in = 17,500 N/m.

[1]The data in this table is intended to be used with the design provisions of ACI 318-14 Chapter 17 or ACI 318-11 Appendix D, as applicable; for anchors resisting seismic load combinations, the additional requirements of ACI 318-14 17.2.3 or ACI 318-11 D.3.3, as applicable, shall apply.
[2]Installation must comply with the manufacturers printed installation instructions and details.
[3]The 1/2- and 5/8-inch diameter Trubolt + Wedge Anchors are ductile steel elements as defined by ACI 318-14 2.3 or ACI 318-11 D.1, as applicable.
[4]All values of ϕ apply to the load combinations of IBC Section 1605.2, ACI 318-14 Section 5.3 or ACI 318-11 Section 9.2, as applicable. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of ϕ must be determined in accordance with ACI 318-11 D.4.4. For installations where reinforcement that complies with ACI 318-14 Chapter 17 or ACI 318-11 Appendix D, as applicable, requirements for Condition A is present, the appropriate ϕ factor must be determined in accordance with ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable.
[5]The actual diameter for the 5/8" diameter anchor is 0.615-inch.
[6]Steel strength in shear values are based on test results per ACI 355.2, Section 9.4 and must be used for design.
[7]The notation in brackets is for the 2006 IBC.

TABLE 7—EXAMPLE ITW RED HEAD CARBON STEEL TRUBOLT+ WEDGE ANCHOR AND OVERHEAD TRUBOLT+ WEDGE ANCHOR ALLOWABLE STRESS DESIGN (ASD) VALUES FOR ILLUSTRATIVE PURPOSES[1,2,3,4,5,6,7,8,9,10]

ANCHOR NOTATION	ANCHOR EMBEDMENT DEPTH (inches), h_{nom}	EFFECTIVE EMBEDMENT DEPTH (inches), h_{ef}	ALLOWABLE TENSION LOAD (lbs)
$^3/_8$	2	$1^5/_8$	1,090
$^1/_2$	$2^1/_2$	2	1,490
	$3^3/_4$	$3^1/_4$	2,870
$^5/_8$	$3^1/_4$	$2^3/_4$	2,385
	$4^3/_4$	$4^1/_4$	3,910
$^3/_4$	$4^3/_8$	$3^3/_4$	3,825

For **SI**: 1 inch = 25.4 mm, 1 lbf = 4.45 N.

Design assumptions:

[1]Single anchor with static tension load only.
[2]Concrete determined to remain uncracked for the life of the anchorage.
[3]Load combinations are in accordance with ACI 318-14 Section 5.3 or ACI 318-11 Section 9.2, as applicable, and no seismic loading.
[4]30 percent dead load and 70 percent live load, controlling load combination 1.2*D* + 1.6*L*.
[5]Calculation of weighted average for α: 1.2*D* + 1.6*L* = 1.2(0.3) + 1.6(0.7) = 1.48.
[6]f'_c = 2,500 psi (normal-weight concrete).
[7]$c_{a1} = c_{a2} >= c_{ac}$.
[8]$h \geq h_{min}$.
[9]Values are for Condition B where supplementary reinforcement in accordance with ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable, is not provided.
[10]The Overhead Trubolt+ Wedge version is available in a carbon steel $^3/_8$-inch-diameter only.

Illustrative Procedure to Calculate Allowable Stress Design Tension Value:

RED HEAD Carbon Steel Trubolt+ Wedge Anchor $^1/_2$ inch diameter using an effective embedment of $3^1/_4$ inches, assuming the given conditions in Table 3, in accordance with ACI 318-14 Chapter 17, ACI 318-11 Appendix D and this report.

	PROCEDURE		CALCULATION
Step 1	Calculate steel strength of a single anchor in tension per ACI 318-14 17.4.1.2, ACI 318-11 D 5.1.2, Table 3 of this report	ϕN_{sa}	$= \phi N_{sa}$ =0.75*8,925 **=6,694 lbs steel strength**
Step 2	Calculate concrete breakout strength of a single anchor in tension per ACI 318-14 17.4.2.1, ACI 318-11 D 5.2.1, Table 3 of this report	N_b	$= k_{uncr} * \lambda_a * \sqrt{f'_c} * h_{ef}^{1.5}$ $= 24 * 1.0 * \sqrt{2{,}500} * 3.25^{1.5}$ = 7,031 lbs
		ϕN_{cb}	$= \phi A_{NC}/A_{NC0}\, \psi_{ed,N}\, \psi_{c,N}\, \psi_{cp,N}\, N_b$ = 0.65*(95/95)/1.0*1.0*1.0*7,031 = 0.65*7,031 **= 4,570 lbs concrete breakout strength**
Step 3	Calculate pullout strength in tension per ACI 318-14 17.4.3.2, ACI 318-11 D 5.3.2, Table 3 of this report	ϕN_{pn}	$= \phi N_{p,uncr}\, \psi_{c,P}\, (f'_{c,act}/2{,}500)^n$ $= 0.65*6{,}540*1.0*1.0^{0.5}$ **= 4,251 lbs pullout strength**
Step 4	Determine controlling resistance strength in tension per ACI 318-14 17.3.1.1, ACI 318-11 D 4.1.1		**= 4,251 lbs controlling resistance**
Step 5	Calculate allowable stress design conversion factor for loading condition per ACI 318-14 Section 5.3, ACI 318-11 Section 9.2	α	=1.2*D* + 1.6*L* =1.2(0.3) + 1.6(0.7) **=1.48**
Step 6	Calculate allowable stress design value per Section 4.2 of this report	$T_{allowable,ASD}$	$= \phi N_n / \alpha$ = 4,251 / 1.48 **= 2,870 lbs allowable stress design**

TABLE 8—ITW RED HEAD CARBON STEEL TRUBOLT+ WEDGE ANCHOR AND OVERHEAD TRUBOLT+ WEDGE ANCHOR INSTALLATION INFORMATION FOR ANCHORS LOCATED IN THE SOFFIT OF CONCRETE FILL ON METAL DECK FLOOR AND ROOF ASSEMBLIES[1,3]

PARAMETER	SYMBOL	UNITS	NOMINAL ANCHOR DIAMETER (inch)				
			$^3/_8$	$^1/_2$		$^5/_8$	
Anchor outer diameter	$d_a[d_o]^2$	inches	0.361	0.5		0.615	
Nominal carbide bit diameter	d_{bit}	inches	$^3/_8$	$^1/_2$		$^5/_8$	
Location of Installation	-	-	upper and lower flute	upper and lower flute	lower flute	lower flute	
Minimum effective embedment depth	h_{ef}	Inches	$1^5/_8$	2	$3^1/_4$	$2^3/_4$	$4^1/_4$
Anchor embedment depth	h_{nom}	Inches	2	$2^1/_2$	$3^3/_4$	$3^1/_4$	$4^3/_4$
Minimum hole depth	h_o	Inches	$2^1/_4$	$2^3/_4$	4	$3^1/_2$	5
Minimum overall anchor length	l_{anchor}	Inches	$2^1/_2$	$3^3/_4$	$4^1/_2$	$4^1/_4$	6
Installation torque	T_{inst}	ft-lb	30	45		90	
Minimum diameter of hole in fastened part	d_h	inches	$^1/_2$	$^5/_8$		$^3/_4$	

For **SI:** 1 inch = 25.4 mm, 1 ft-lb = 1.356 N-m.

[1]Anchors in the lower and upper flute may be installed with a maximum 1-inch offset in either direction from the center of the flute. In addition, anchors must have an axial spacing along the flute equal to the greater of $3h_{ef}$ or 1.5 times the flute width.
[2]The notation in brackets is for the 2006 IBC.
[3]The Overhead Trubolt+ Wedge version is available in a carbon steel $^3/_8$-inch-diameter only.

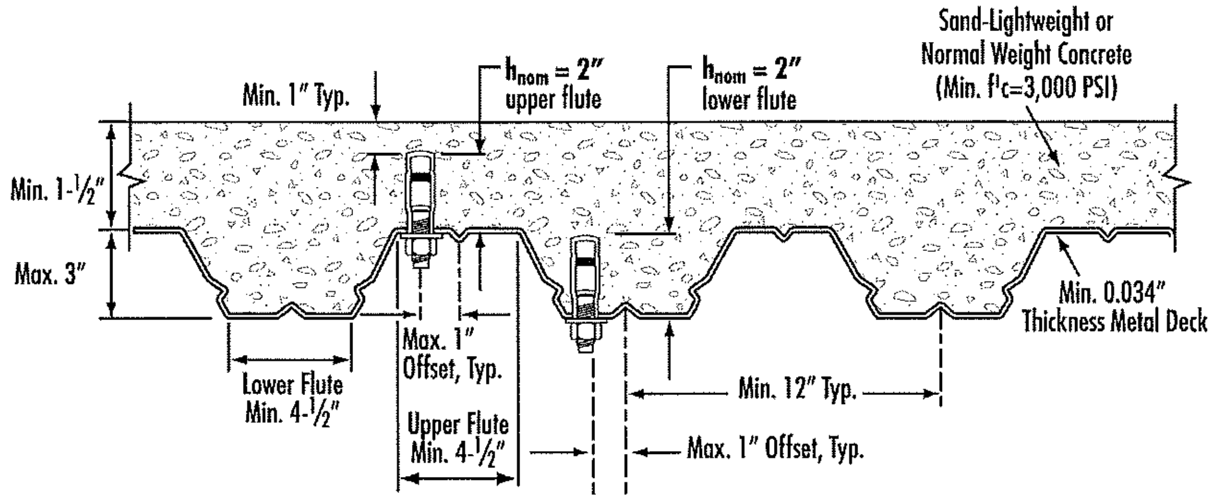
TABLE 9—ITW RED HEAD CARBON STEEL TRUBOLT+ WEDGE ANCHOR AND OVERHEAD TRUBOLT+ WEDGE ANCHOR DESIGN INFORMATION FOR ANCHORS LOCATED IN THE SOFFIT OF CONCRETE FILL ON METAL DECK FLOOR AND ROOF ASSEMBLIES[1,2]

CHARACTERISTIC	SYMBOL	UNITS	NOMINAL ANCHOR DIAMETER (inch)				
			$^3/_8$	$^1/_2$		$^5/_8$	
Location of Installation	-	-	upper and lower flute	upper and lower flute	lower flute	lower flute	
Minimum effective embedment depth	h_{ef}	inches	$1^5/_8$	2	$3^1/_4$	$2^3/_4$	$4^1/_4$
Characteristic pullout strength, uncracked concrete over metal deck	$N_{p,deck,uncr}^1$	lbf	2,170	2,515	5,285	3,365	6,005
Characteristic pullout strength, cracked concrete over metal deck	$N_{p,deck,cr}^1$	lbf	1,650	1,780	4,025	2,405	5,025
Characteristic shear strength, concrete over metal deck	$V_{sa,deck}^1$	lbf	$1,640^3$	2,200	3,790	2,890	6,560
Reduction factor for pullout strength in tension, Condition B	ϕ	-	0.65	0.65		0.65	
Reduction factor for steel strength in shear, Condition B	ϕ	-	0.60	0.65		0.65	

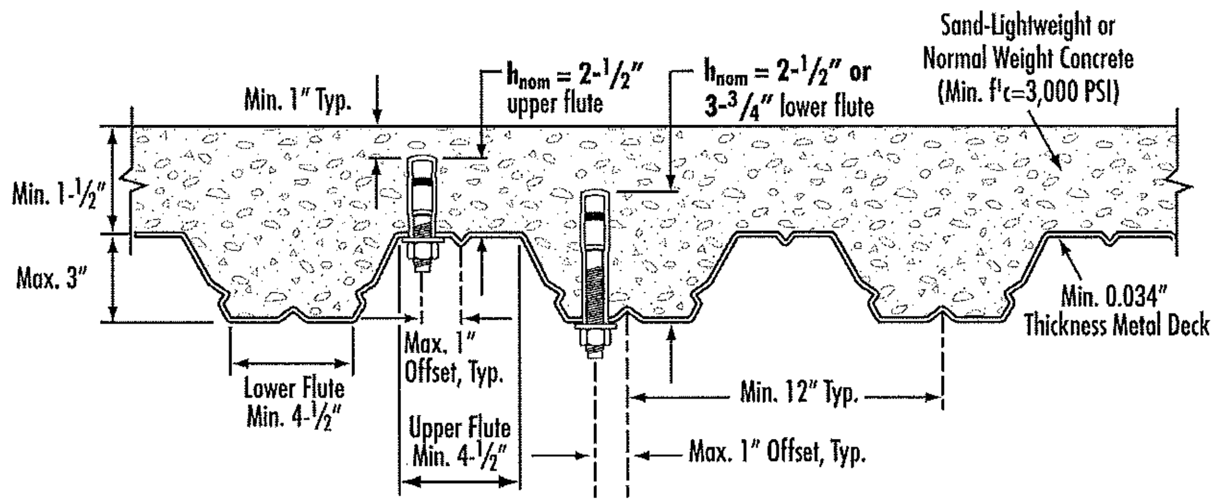
For **SI:** 1 inch = 25.4 mm, 1 lbf = 4.45 N.

[1]Values for $N_{p,deck}$ and $V_{sa,deck}$ apply to sand-lightweight concrete having a minimum concrete compressive strength, f'_c, of 3,000 psi [minimum of 24 MPa is requiredunder ADIBC Appendix L, Section 5.1.1].
[2]All values of ϕ apply to the load combinations of IBC Section 1605.2, ACI 318-14 Section 5.3 or ACI 318-11 Section 9.2. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of ϕ must be determined in accordance with ACI 318-11 D.4.4. For installations where reinforcement that complies with ACI 318-14 Chapter 17 or ACI 318-11 Appendix D, as applicable, requirements for Condition A is present, the appropriate ϕ factor must be determined in accordance with ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable.
[3]For seismic applications according to Section 4.10.3 of this report multiply the value of $V_{sa,deck}$ by 0.84.

Nominal Anchor Diameter = 3/8″



Nominal Anchor Diameter = 1/2″



Nominal Anchor Diameter = 5/8″

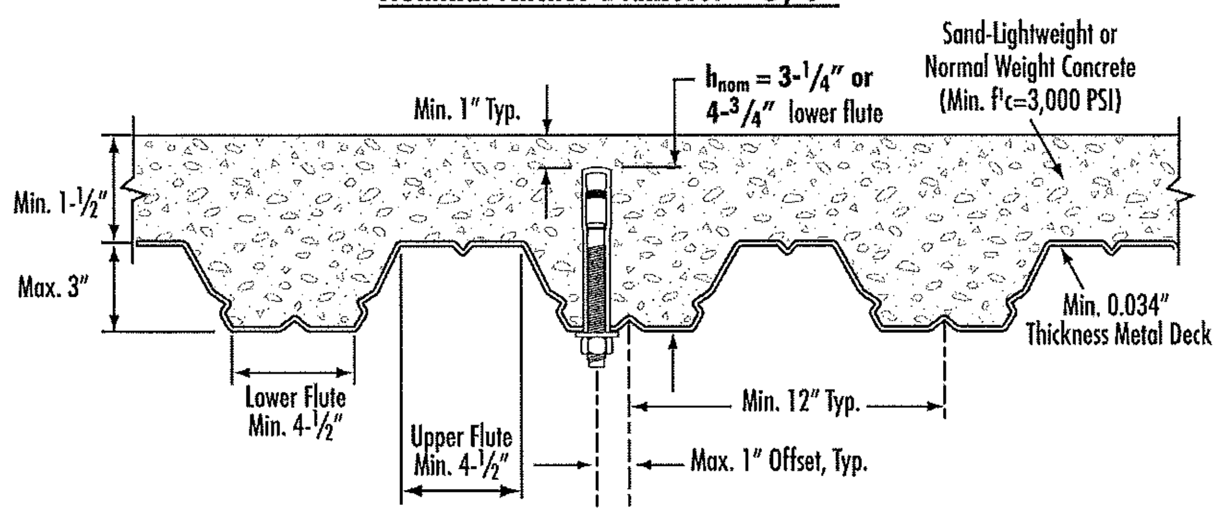


FIGURE 7—ITW RED HEAD CARBON STEEL TRUBOLT+ WEDGE ANCHOR AND OVERHEAD` TRUBOLT+ WEDGE ANCHOR LOCATED IN THE SOFFIT OF CONCRETE OVER STEEL DECK FLOOR AND ROOF ASSEMBLIES (1 inch = 25.4 mm)

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ICC-ES Evaluation Report

ESR-2427 FBC Supplement

Reissued November 2016

This report is subject to renewal November 2017.

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DIVISION: 03 00 00—CONCRETE
Section: 03 16 00—Concrete Anchors

DIVISION: 05 00 00—METALS
Section: 05 05 19—Post-installed Concrete Anchors

REPORT HOLDER:

ITW RED HEAD
700 HIGH GROVE BOULEVARD
GLENDALE HEIGHTS, ILLINOIS 60139
(800) 848-5611
www.itw-redhead.com
techsupport@itwccna.com

EVALUATION SUBJECT:

ITW RED HEAD CARBON STEEL TRUBOLT+ WEDGE ANCHORS, STAINLESS STEEL TRUBOLT+ WEDGE ANCHORS AND CARBON STEEL OVERHEAD TRUBOLT+ WEDGE ANCHORS FOR CRACKED AND UNCRACKED CONCRETE

1.0 REPORT PURPOSE AND SCOPE

Purpose:

The purpose of this evaluation report supplement is to indicate that ITW Red Head Trubolt+ Wedge Anchors and OVERHEAD Trubolt+ Wedge Anchors for Cracked and Uncracked Concrete, recognized in ICC-ES master evaluation report ESR-2427, have also been evaluated for compliance with the codes noted below.

Applicable code editions:

- 2014 and 2010 *Florida Building Code—Building*
- 2014 and 2010 *Florida Building Code—Residential*

2.0 CONCLUSIONS

The ITW Red Head Trubolt+ Wedge Anchors and OVERHEAD Trubolt+ Wedge Anchors for Cracked and Uncracked Concrete, described in Sections 2.0 through 7.0 of the master evaluation report ESR-2427, comply with the 2014 and 2010 *Florida Building Code—Building* and the 2014 and 2010 *Florida Building Code—Residential*, provided the design and installation are in accordance with the 2012 *International Building Code®* provisions noted in the master report and the following provisions apply:

- Design wind loads must be based on Section 1609 of the 2014 and 2010 *Florida Building Code—Building* or Section 301.2.1.1 of the 2014 and 2010 *Florida Building Code—Residential*, as applicable.
- Load combinations must be in accordance with Section 1605.2 or Section 1605.3 of the 2014 and 2010 *Florida Building Code—Building*, as applicable.

Use of the ITW Red Head Trubolt+ Wedge Anchors and OVERHEAD Trubolt+ Wedge Anchors for Cracked and Uncracked Concrete for compliance with the High-Velocity Hurricane Zone provisions of the 2014 and 2010 *Florida Building Code—Building* and the 2014 and 2010 *Florida Building Code—Residential* has not been evaluated, and is outside the scope of this supplemental report.

For products falling under Florida Rule 9N-3, verification that the report holder's quality-assurance program is audited by a quality-assurance entity approved by the Florida Building Commission for the type of inspections being conducted is the responsibility of an approved validation entity (or the code official when the report holder does not possess an approval by the Commission).

This supplement expires concurrently with the master report, reissued November 2016.

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Table 12.2A Lag Screw Reference Withdrawal Design Values, W[1]

Tabulated withdrawal design values (W) are in pounds per inch of thread penetration into side grain of wood member. Length of thread penetration in main member shall not include the length of the tapered tip (see 12.2.1.1).

Specific Gravity, G[2]	Lag Screw Diameter, D										
	1/4"	5/16"	3/8"	7/16"	1/2"	5/8"	3/4"	7/8"	1"	1-1/8"	1-1/4"
0.73	397	469	538	604	668	789	905	1016	1123	1226	1327
0.71	381	450	516	579	640	757	868	974	1077	1176	1273
0.68	357	422	484	543	600	709	813	913	1009	1103	1193
0.67	349	413	473	531	587	694	796	893	987	1078	1167
0.58	281	332	381	428	473	559	641	719	795	869	940
0.55	260	307	352	395	437	516	592	664	734	802	868
0.51	232	274	314	353	390	461	528	593	656	716	775
0.50	225	266	305	342	378	447	513	576	636	695	752
0.49	218	258	296	332	367	434	498	559	617	674	730
0.47	205	242	278	312	345	408	467	525	580	634	686
0.46	199	235	269	302	334	395	453	508	562	613	664
0.44	186	220	252	283	312	369	423	475	525	574	621
0.43	179	212	243	273	302	357	409	459	508	554	600
0.42	173	205	235	264	291	344	395	443	490	535	579
0.41	167	198	226	254	281	332	381	428	473	516	559
0.40	161	190	218	245	271	320	367	412	455	497	538
0.39	155	183	210	236	261	308	353	397	438	479	518
0.38	149	176	202	227	251	296	340	381	422	461	498
0.37	143	169	194	218	241	285	326	367	405	443	479
0.36	137	163	186	209	231	273	313	352	389	425	460
0.35	132	156	179	200	222	262	300	337	373	407	441
0.31	110	130	149	167	185	218	250	281	311	339	367

1. Tabulated withdrawal design values, W, for lag screw connections shall be multiplied by all applicable adjustment factors (see Table 11.3.1).
2. Specific gravity, G, shall be determined in accordance with Table 12.3.3A.

12.2.3.2 For calculation of the fastener reference withdrawal design value in pounds, the unit reference withdrawal design value in lbs/in. of fastener penetration from 12.2.3.1 shall be multiplied by the length of fastener penetration, p_t, into the wood member.

12.2.3.3 The reference withdrawal design value, in lbs/in. of penetration, for a single post-frame ring shank nail driven in the side grain of the main member, with the nail axis perpendicular to the wood fibers, shall be determined from Table 12.2D or Equation 12.2-4, within the range of specific gravities and nail diameters given in Table 12.2D. Reference withdrawal design values, W, shall be multiplied by all applicable adjustment factors (see Table 11.3.1) to obtain adjusted withdrawal design values, W'.

$$W = 1800\, G^2\, D \qquad (12.2\text{-}4)$$

12.2.3.4 For calculation of the fastener reference withdrawal design value in pounds, the unit reference withdrawal design value in lbs/in. of ring shank penetration from 12.2.3.3 shall be multiplied by the length of ring shank penetration, p_t, into the wood member.

12.2.3.5 Nails and spikes shall not be loaded in withdrawal from end grain of wood (C_{eg}=0.0).

12.2.3.6 Nails, and spikes shall not be loaded in withdrawal from end-grain of laminations in cross-laminated timber (C_{eg}=0.0).

12.2.4 Drift Bolts and Drift Pins

Reference withdrawal design values, W, for connections using drift bolt and drift pin connections shall be determined in accordance with 11.1.1.3.

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Table 12K LAG SCREWS: Reference Lateral Design Values, Z, for Single Shear (two member) Connections[1,2,3,4]

for sawn lumber or SCL with ASTM A653, Grade 33 steel side plate (for t_s<1/4") or ASTM A 36 steel side plate (for t_s=1/4")
(tabulated lateral design values are calculated based on an assumed length of lag screw penetration, p, into the main member equal to 8D)

Side Member Thickness	Lag Screw Diameter	G=0.67 Red Oak		G=0.55 Mixed Maple Southern Pine		G=0.5 Douglas Fir-Larch		G=0.49 Douglas Fir-Larch (N)		G=0.46 Douglas Fir(S) Hem-Fir(N)		G=0.43 Hem-Fir		G=0.42 Spruce-Pine-Fir		G=0.37 Redwood (open grain)		G=0.36 Eastern Softwoods Spruce-Pine-Fir(S) Western Cedars Western Woods		G=0.35 Northern Species	
t_s in.	D in.	$Z_{\parallel}$ lbs.	$Z_{\perp}$ lbs.	$Z_{\parallel}$ lbs.	$Z_{\perp}$ lbs.	$Z_{\parallel}$ lbs.	$Z_{\perp}$ lbs.	$Z_{\parallel}$ lbs.	$Z_{\perp}$ lbs.	$Z_{\parallel}$ lbs.	$Z_{\perp}$ lbs.	$Z_{\parallel}$ lbs.	$Z_{\perp}$ lbs.	$Z_{\parallel}$ lbs.	$Z_{\perp}$ lbs.	$Z_{\parallel}$ lbs.	$Z_{\perp}$ lbs.	$Z_{\parallel}$ lbs.	$Z_{\perp}$ lbs.	$Z_{\parallel}$ lbs.	$Z_{\perp}$ lbs.
0.075	1/4	170	130	160	120	150	110	150	110	160	100	140	100	140	100	130	90	130	90	130	90
(14 gage)	5/16	220	160	200	140	190	130	190	130	190	130	180	120	180	120	170	110	170	110	160	100
	3/8	220	160	200	140	200	130	190	130	190	120	180	120	180	120	170	110	170	100	170	100
0.105	1/4	180	140	170	130	160	120	160	120	160	110	150	110	150	110	140	100	140	100	140	90
(12 gage)	5/16	230	170	210	150	200	140	200	140	190	130	190	130	190	120	160	110	170	110	170	110
	3/8	230	160	210	140	200	140	200	130	200	130	190	120	190	120	180	110	160	110	170	110
0.120	1/4	190	150	180	130	170	120	170	120	160	120	160	110	160	110	150	100	150	100	140	100
(11 gage)	5/16	230	170	210	150	210	140	200	140	200	140	190	130	190	130	180	120	180	120	180	110
	3/8	240	170	220	150	210	140	210	140	200	130	200	130	190	120	160	110	180	110	180	110
0.134	1/4	200	150	180	140	180	130	170	130	170	120	160	120	160	110	150	110	150	100	150	100
(10 gage)	5/16	240	160	220	160	210	150	210	140	200	140	200	130	200	130	190	120	180	120	180	120
	3/8	240	170	220	150	220	140	210	140	210	140	200	130	200	130	190	120	190	120	180	110
0.179	1/4	220	170	210	160	200	150	200	140	190	140	190	130	190	130	180	120	170	120	170	120
(7 gage)	5/16	260	190	240	170	230	160	230	160	230	150	220	150	220	150	210	130	200	130	200	130
	3/8	270	190	260	170	240	160	240	160	230	150	220	140	220	140	210	130	210	130	200	130
0.239	1/4	240	180	220	160	210	150	210	150	200	140	190	140	190	130	180	120	180	120	180	120
(3 gage)	5/16	300	220	280	190	270	180	260	180	260	170	250	160	250	160	230	150	230	150	230	140
	3/8	310	220	280	190	270	180	270	180	260	170	250	160	250	160	240	140	230	140	230	140
	7/16	420	290	390	260	380	240	370	240	360	230	350	220	350	220	330	200	330	200	320	190
	1/2	510	340	470	300	460	290	450	280	440	270	430	260	420	260	400	240	400	230	390	230
	5/8	770	490	710	430	680	400	680	400	660	380	640	370	630	360	600	330	590	330	580	320
	3/4	1110	670	1020	590	960	560	970	560	960	530	920	500	910	500	860	450	850	450	840	440
	7/8	1510	880	1390	780	1330	730	1320	710	1280	690	1250	650	1230	650	1170	590	1100	580	1140	570
	1	1940	1100	1760	980	1710	910	1700	890	1650	860	1600	820	1690	810	1500	740	1400	730	1460	710
1/4	1/4	240	180	220	160	210	150	210	150	200	140	200	140	190	130	180	120	180	120	180	120
	5/16	310	220	260	200	270	180	270	180	260	170	250	170	250	160	230	150	230	150	230	140
	3/8	320	220	290	190	280	180	270	160	270	170	260	160	250	160	240	150	240	140	230	140
	7/16	480	320	440	280	420	270	420	260	410	250	390	240	390	230	370	220	360	210	360	210
	1/2	580	390	540	340	520	320	510	320	500	310	480	290	480	290	460	270	450	260	440	260
	5/8	850	530	780	470	750	440	740	440	720	420	700	400	690	400	660	370	650	360	640	350
	3/4	1200	730	1100	640	1060	600	1050	590	1020	570	990	540	980	530	930	490	920	480	900	470
	7/8	1600	930	1470	820	1410	770	1400	760	1360	720	1320	690	1310	680	1240	630	1220	620	1200	600
	1	2040	1150	1870	1000	1800	950	1780	930	1730	900	1680	850	1680	840	1570	770	1550	780	1630	740

1. Tabulated lateral design values, Z, shall be multiplied by all applicable adjustment factors (see Table 11.3.1).
2. Tabulated lateral design values, Z, are for "reduced body diameter" lag screws (see Appendix Table L2) inserted in side grain with screw axis perpendicular to wood fibers; screw penetration, p, into the main member equal to 8D; dowel bearing strengths, F_e, of 61,850 psi for ASTM A653, Grade 33 steel and 87,000 psi for ASTM A36 steel and screw bending yield strengths, F_{yb}, of 70,000 psi for D = 1/4", 60,000 psi for D = 5/16", and 45,000 psi for D ≥3/8".
3. Where the lag screw penetration, p, is less than 8D but not less than 4D, tabulated lateral design values, Z, shall be multiplied by p/8D or lateral design values shall be calculated using the provisions of 12.3 for the reduced penetration.
4. The length of lag screw penetration, p, not including the length of the tapered tip, E (see Appendix Table L2), of the lag screw into the main member shall not be less than 4D. See 12.1.4.6 for minimum length of penetration, p_{min}.